ALTERNATIVE STORMWATER MANAGEMENT FACILITY REPORT



I-75 (SR 93) PD&E Study

From North of SR 52 to South of CR 476B (Pasco, Hernando, and Sumter Counties) FAP No.: 0751-120I WPI No.: 411014 1 22 01

June 2007



Florida Department of Transportation District Seven

ALTERNATIVE STORMWATER MANAGEMENT FACILITY REPORT

I-75 (SR 93)

FROM NORTH OF SR 52 TO SOUTH OF CR 476B PASCO, HERNANDO AND SUMTER COUNTIES, FLORIDA

W.P.I. Segment Number: 411014 1

The proposed action consists of upgrading I-75 from a four-lane to an eight-lane, divided, rural interstate highway for approximately 20.8 miles.

Florida Department of Transportation District Seven

Tampa, Florida

Prepared By: H.W. Lochner, Inc. Clearwater, FL

June 2007

EXECUTIVE SUMMARY

The Florida Department of Transportation (FDOT) has conducted a Project Development and Environment (PD&E) Study to document the preliminary engineering concept of the I-75 project corridor from north of SR 52 in Pasco County to south of CR 476B in Sumter County, a distance of approximately 20.8 miles. The purposes of the PD&E Study are to develop engineering and environmental data and document information which will aid the FDOT and the Federal Highway Administration in determining the type, design, and location of the proposed improvements, and the impacts, if any, associated with the recommended alignment.

The Alternative Stormwater Management Facility (SMF) Report identifies SMF alternatives and floodplain compensation (FPC) sites and includes an alternative analysis for selection of a preferred alternative for the PD&E Study. This study analyzes SMF alternatives that are hydraulically feasible and environmentally permittable based on the best available information. These alternatives were then compared based on Section 4(f) involvement; cultural resources; environmental impacts including wetlands, upland habitat and protected species involvement; petroleum and hazardous materials contamination; and economic factors including right-of-way costs.

Preferred SMF / FPC Sites	Station - Location	Area (ac)
SMF 2A	1255+00, RT	2.7
SMF 3C	1281+00, RT	2.2
FPC 3A	1284+00, LT	0.7
SMF 4B	1298+00, RT	4.1
SMF 5C	1339+00, RT	4.3
FPC (Adjacent to SMF) 5C	1339+00, RT	0.2
SMF 6C	1380+00, LT	3.1
SMF 7C	1404+00, LT	1.5
SMF 8A	1420+00, LT	2.2
SMF 9A	1426+00, LT	2.2
FPC 9C	1428+00, LT	0.7
SMF 10B	1454+00, LT	4.1
SMF 11B	1483+00, RT	1.7
SMF 12A	1499+00, LT	1.7
SMF 13C	1548+00, LT	5.1
SMF 14C	1580+00, RT	2.1

The preferred SMF and FPC sites are listed in the table below.

Preferred SMF / FPC Sites	Station - Location	Area (ac)	
SMF 15A	1595+00, RT	7.8	
SMF 16A	1612+00, LT & RT	7.9	
SMF 17B	1691+00, LT	8.7	
SMF 18B	1707+00, RT	7.5	
FPC 18A	1698+00, RT	3.0	
SMF 19B	1763+00, LT	7.8	
SMF 20B	1794+00, LT	4.7	
SMF 21B	1825+00, RT	6.7	
SMF 22A	1862+00, LT	7.1	
SMF 23A	1896+00, RT	4.8	
SMF 24B	1933+00, LT	7.1	
SMF 25C	1987+00, LT	5.1	
SMF 26C	2006+00, RT	5.2	
SMF 27C	2028+00, RT	3.7	
SMF 29C	2068+00, RT	7.6	
SMF 30B	2162+00, RT	15.0	
SMF 31D, 4b(e)C & 4b(w)C	2200+00, RT; 2233+00, RT & 2233+00, LT	13.9 (20.4 Easement) ⁽¹⁾	
5aC & 5bC	2252+00, LT & 2265+00, LT	12.0 (53.1 Easement) ⁽¹⁾	
6a/bC & 6cC	2300+00, RT & 2339+00, RT	9.4 (37.3 Easement) ⁽¹⁾	
7C	2345+00, LT	7.4 (10.8 Easement) ⁽¹⁾	

(1) Department intends to acquire Perpetual Transportation/Drainage/Maintenance Easements within the Withlacoochee State Forest. These easements will include areas of stomwater conveyance.

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1.0 INTRODUCTION

The Florida Department of Transportation (FDOT) has conducted a Project Development and Environment (PD&E) Study to document the preliminary engineering concept of the I-75 project corridor from north of SR 52 in Pasco County to south of CR 476B in Sumter County. The purposes of the PD&E Study are to develop engineering and environmental data and document information which will aid the FDOT and the Federal Highway Administration (FHWA) in determining the type, design, and location of the proposed improvements, and the impacts, if any, associated with the recommended alignment.

The Alternative Stormwater Management Facility (SMF) Report identifies SMF alternatives and includes an alternative analysis for selection of a preferred alternative for the PD&E Study. This study analyzes SMF alternatives that are hydraulically feasible and environmentally permittable based on the best available information. These alternatives were then compared based on Section 4(f) involvement; cultural resources; environmental impacts including wetlands, upland habitat and protected species involvement; petroleum and hazardous materials contamination; and economic factors including right-of-way costs. An alternatives evaluation matrix that summarizes the comparative analysis was developed and is shown in Tables 5 through 36 of Section 8.0. The process of defining and developing the information base included the following:

- FEMA Flood Insurance Rate Maps (FIRM) for Pasco County, November 18, 1981 and September 30, 1982, FIRMs for Hernando County, April 17, 1984, and FIRMs for Sumter County, March 15, 1982.
- United States Department of Agriculture, Soil Conservation Service (now Natural Resource Conservation Service), <u>Soil Survey of Pasco County</u>, Florida, June 1982; <u>Soil Survey of Hernando County</u>, Florida, July 1977, and <u>Soil Survey of Sumter</u> <u>County</u>, Florida, 1988.
- United States Geological Survey (USGS) Quadrangle Maps, Scale 1:24,000: San Antonio, FLA, 1954 (Photo revised 1988); Spring Lake, FLA, 1954 (Photo revised 1988); Saint Catherine, FLA, 1958 and Lacoochee, FLA, 1960 (Photo revised 1988).
- Southwest Florida Water Management District (SWFWMD), Aerial Photography With Contours, Scale 1"=200', 1-foot contour interval, January 1973, October/December 1985, February 1987, December 1987, April 1984, and November 1991.
- Straight Line Diagram (SLD) for I-75, FDOT District Seven, Planning and Statistics Office, December 1, 2004 for Pasco County, January 31, 2005 for Hernando County, and May 8, 2004 for Sumter County.
- FDOT Drainage Manual, October 2004.

2.0 **PROJECT DESCRIPTION**

The study area for this project extends from just north of SR 52 in Pasco County, through Hernando County, to just south of County Road (CR) 476B in Sumter County, Florida; a distance of approximately 20.8 miles. The study area for this project consists of the mainline of I-75 and the area bordering it for the assessment of social, economic, and cultural effects.

Presently, within the project limits, I-75 is a four-lane, median divided, limited access, rural highway that generally occupies a 300-foot wide band right-of-way. No major improvements have been made to this segment of I-75 since its original construction in the 1960s. The study area, in addition to the mainline of I-75, includes two interchanges and two rest areas (one in each direction). More specifically, a partial cloverleaf interchange is currently provided at Blanton Road (CR 41) approximately 6.3 miles north of SR 52 in Pasco County and a diamond interchange is present at Cortez Road (SR 50/US 98), approximately 9.3 miles north of CR 41 in Hernando County. The rest areas are located approximately 4.9 miles north of SR 50 in Sumter County.

From north of SR 50 to the northern terminus of the project, Withlacoochee State Forest abuts the entire western border of I-75 and most of its eastern border. At the Hernando/Sumter County line, approximately 1.5 miles from the northern project terminus, I-75 crosses the Withlacoochee River. The project location is shown in Figure 1.

3.0 EXISTING CONDITIONS

3.1 Existing Roadway Conditions

The existing roadway is typically a four-lane rural facility with two 12-foot lanes in each direction, a 64-foot depressed median with an 8-foot inside shoulder (4 feet paved) and 12-foot outside shoulders (10 feet paved). The roadway cross section varies throughout the length of the project. The posted speed limit is 70 mph. These features are provided within a right-of-way that is predominantly 300 feet wide except at certain locations where northbound and southbound I-75 follow independent alignments. The existing roadway typical section for I-75 is shown in Figure 2.

3.2 Existing Drainage Conditions

3.2.1 Topography and Hydrologic Features

In Pasco County, the Withlacoochee, Hillsborough, Pithlachascotee and Anclote Rivers are the major waterways (USDA 1982:5). In addition, over 190 lakes are located throughout Pasco County, including Lake Iola, Moody Lake, and Mud Lake near the I-75 corridor. Stanley Branch, Bee Tree Branch, and Cypress Creek also cross the project corridor. Hernando County is situated within the Middle Gulf Hydrologic System (Cherry et al. 1970). The major and permanent streams are the Withlacoochee, Little Withlacoochee, and Weeki Wachee Rivers (USDA 1977). Numerous small streams and creeks are located in the coastal areas. Springs also are common along the coast. Of the approximate 130 lakes scattered throughout Hernando County, those located proximate to the I-75 corridor include McClendon, Robinson, and Oriole Lakes. During the Late Pleistocene/Early Holocene, many of these water features were non-existent. The Withlacoochee and Little Withlacoochee Rivers also flow through part of Sumter County. The former forms the line dividing Hernando and Sumter Counties. Several waterways, including the Dead River, Outlet River, and Jumper Creek discharge into the Withlacoochee River (USDA 1988:2). Wild Cow Prairie, another wetland feature near the project area, is situated at the northern end of the project area. Elevations throughout the project corridor range from about 59 feet







National Geodetic Vertical Datum (NGVD) of 1929 at the northern end of the project to about 193 feet in the middle of the project. Table 1 describes the regional drainage boundaries as well as the basin boundary limits for the sub-basins for this project. A review of the best available information listed in Section 1.0 of this report in addition to field reconnaissance was conducted to assess the sub-basin limits. The table also includes the existing cross drains.

Regional Drainage Basin	Regional Sub- Basins	Basin Boundaries	Draining to Cross Drain No.
	1	North of SR 52 to Oscie Murphy Rd.	N/A
	2	Oscie Murphy Rd. to Sta. 1265+45	1
	3	Sta. 1265+45 to Sta.1292+50	2
	4	Sta. 1292+50 to Sta. 1330+75	3
	5	Sta. 1330+75 to Sta. 1363+10	N/A
	6	Sta. 1363+10 to Sta. 1393+35	4
	7	Sta. 1393+35 to Sta. 1405+75	5
	8	Sta. 1405+75 to Sta. 1424+10	N/A
	9	Sta. 1424+10 to Sta. 1444+55	6, 7
	10	Sta. 1444+55 to Sta. 1483+50	8
Hillsborough	11	Sta. 1483+50 to Sta. 1496+85	9
River Basin	12	Sta. 1496+85 to Sta. 1510+00	10
	13	Sta. 1510+00 to Sta. 1564+10	11, 12
	14	Sta. 1564+10 to Sta. 1588+55	N/A
	15	Sta. 1588+55 to Sta. 1607+35	13, 14
	16	Sta. 1607+35 to Sta. 1644+05	15, 16
	17	Sta. 1644+05 to Sta. 1694+75	17
	18	Sta. 1694+75 to Sta. 1735+90	18,19,20
	19	Sta. 1735+90 to Sta. 1779+35	21, 22
	20	Sta. 1779+35 to Sta. 1801+50	23, 24, 25
	21	Sta. 1801+50 to Sta. 1845+00	26, 27
	22	Sta. 1845+00 to Sta. 1883+45	28, 29
Withlacoochee	23	Sta. 1883+45 to Sta. 1915+00	30
River Basin	24	Sta. 1915+00 to Sta. 1961+25	31, 32, 33, 34
	25	Sta. 1961+25 to Sta. 1987+60	35

Table 1Regional Drainage Boundaries

Regional Drainage Basin	Regional Sub- Basins	Basin Boundaries	Draining to Cross Drain No.
	26	Sta. 1987+60 to Sta. 2016+75	36, 37, 38
Withlacoochee	27	Sta. 2016+75 to Cortez Blvd.	39
River Basin	28	Not Used	N/A
	29a, 29b & 29c	Cortez Blvd. to Sta. 2091+00	40, 41, 42
	30a, 30b, 30c & 30d	Sta. 2091+00 to Sta. 2189+00	43, 44, 45, 46
	31a & 31b	Sta. 2189+00 to Sta. 2239+15	47, 48, 49, 50
	32a & 32b	Sta. 2239+15 to Sta. 2272+50	51, 52, 53
	33a, 33b & 33c	Sta. 2275+25 to Sta. 2332+15	Bridge, 55, 56, 57
	34	Sta. 2332+15 to Sta. 2356+67	58, 59, 60

3.2.2 Existing Drainage Patterns

The I-75 project is within the jurisdiction of the Southwest Florida Water Management District (SWFWMD).

The proposed project is not expected to have an impact on the water quality of the Hillsborough and Withlachochee Rivers. Currently there is little to no treatment of stormwater runoff from I-75. Some treatment is provided by grass swales and wet ditches created during the original construction of the roadway for conveyance of stormwater. Runoff from the bridges enters the waterways directly via scuppers.

There are no existing stormwater retention or detention facilities within the project limits.

3.2.3 Existing Cross Drains

A review of the FDOT construction plans and SLDs indicates that there are sixty existing cross drains within the limits of the I-75 PD&E project. The locations of these drainage structures were verified by field inspection.

Hydraulic equivalency for replacement or modification of the existing cross drains will be determined in subsequent design phases of this project.

The existing cross drains are listed in Table 2. The locations of the existing cross drains are shown on the Concept Plans in Appendix A.

Cross Drain No.	Station	Pipe Size and Material
1	1260+50	8' x 3' CBC
2	1292+75	Bridge CBC
3	1331+00	10' x 5' CBC
4	1378+00	Triple-36" RCP
5	1405+00	36" RCP
6	1424+15	Bridge CBC
7	1425+25	12' x 5' CBC
8	1444+70	6' x 4' CBC
9	1483+70	36" RCP
10	1497+00	36" RCP
11	1518+00	36" RCP
12	1552+85	12' x 12' CBC
13	1604+70	3' x 3' CBC
14	1606+60	36" RCP
15	1628+90	3' x 3' CBC
16	1644+90	36" RCP
17	1680+80	36" RCP
18	1703+20	36" RCP
19	1714+00	24" RCP
20	1730+40	30" RCP
21	1746+00	42" RCP
22	1758+75	48" RCP LT., / 42" RCP RT.
23	1783+75	18" RCP LT. / 3' x 3' CBC RT.
24	1788+50	3' x 3' CBC

Table 2 Existing Cross Drains

Cross Drain No.	Station	Pipe Size and Material
25	1794+60	3' x 3' CBC
26	1803+15	3' x 3' CBC LT. / 10' x 4' CBC RT.
27	1816+85	10' x 4' CBC
28	1853+40	10' X 8' CBC
29	1854+30	18" RCP
30	1899+60	4' x 3' CBC
31	1928+60	15' x 12' CBC
32	1928+80	18" RCP
33	1933+90	24" RCP
34	1939+00	Double 42" RCP
35	1984+00	Double 42" RCP
36	2000+00	30" RCP LT./ 36" RCP RT.
37	2008+05	30" RCP
38	2015+15	24" RCP
39	2031+00	30" RCP
40	2058+75	18" RCP
41	2064+00	36" RCP
42	2075+00	24" RCP LT. / 30" RCP RT.
43	2108+00	30" RCP LT. / 24" RCP RT.
44	2130+00	24" RCP
45	2167+45	30" RCP LT. / 24" RCP RT.
46	2179+00	36" RCP
47	2201+90	24" RCP
48	2226+90	18" RCP
49	2231+90	3' x 3' CBC
50	2235+40	18" RCP

Cross Drain No.	Station	Pipe Size and Material
51	2251+00	18" RCP
52	2254+40	36" RCP
53	2260+90	36" RCP
54	2277+00	Bridge
55	2281+90	18" RCP
56	2284+90	18" RCP
57	2305+90	30" RCP
58	2327+35	18" RCP LT. / 15" RCP RT.
59	2340+90	18" RCP LT. / 15" RCP RT.
60	2348+90	30" RCP

Notes:

CBC - Concrete Box Culvert

RCP - Reinforced Concrete Pipe

4.0 **PROPOSED IMPROVEMENTS**

4.1 **Proposed Typical Sections**

The improvement proposed for I-75 is an eight-lane, divided, rural interstate highway. The widening from four to eight lanes proposes one additional lane in the median and one additional lane to the outside in each direction. Since the remaining median will be 40 feet wide, 24 less than the standard minimum median width for this type of facility, guardrail will be placed along the median and a design variation will be required. The outside border width will also be reduced from 94 feet to 82 feet which will also require a design variation. This typical is shown in Figure 4.

4.2 Recommended Improvement Alternative

The recommended improvement alternative for the I-75 project corridor was developed to avoid or minimize impacts to the surrounding land uses and environmental features. The proposed 8-lane typical section will widen add two additional travel lanes in each direction. These new travel lanes will be added as a widening from the existing pavement.

In general, the existing horizontal and vertical curvature will be retained. Reconstruction of the existing mainline pavement will be necessary at the SR 50 Interchange to construct a new bridge utilizing current vertical curvature design criteria.



Capacity improvements will also be necessary at the CR 41 and SR 50 interchanges. Various ramp alignment shifts will be constructed to accommodate the future increased traffic volumes.

Several bridges carrying minor cross roadways over the interstate must be replaced to accommodate the widened interstate typical section. Varying amounts of approach roadway must be reconstructed to accommodate the new bridge structures.

The recommended improvement alternative is shown on the Concept Plans in Appendix A.

4.3 **Proposed Drainage**

The roadway will primarily be drained by an open drainage system with ditches and swales draining to a SMF. Treatment and attenuation will be provided within wet or dry detention ponds. There will be one preferred SMF site for each basin.

The post-development peak discharge for the 25-year/24-hour rainfall event will not exceed the pre-development peak discharge, in order to comply with SWFWMD regulations. The SMFs will also comply with FDOT Regulation 14.86 to meet critical duration requirements. A pre-application meeting to discuss drainage and floodplain compensation methodology was held with SWFWMD on March 15 2005. The minutes from this meeting are included in Appendix B. See Section 8.1 for SMF sizing methodology and criteria.

5.0 ENVIRONMENTAL EVALUATION

5.1 Jurisdictional Wetland Involvement

Jurisdictional wetlands adjacent to the right-of-way and within the limited access fence were field verified and delineated on aerial mapping. In addition, water conveyance features such as swales and ditches were mapped as well. Only minor impacts to these resources will be impacted by the proposed mainline improvements. These impacts would include culvert extensions and minor modifications to the drainage systems. Depending on the final selection of the preferred stormwater management facilities and floodplain compensation sites, some wetland impacts may occur. However, avoidance and minimization measures will be taken to minimize impacts to these resources. All unavoidable impacts to wetlands will be mitigated for during the permitting process.

5.2 Cultural Features

5.2.1 Section 4(f) Involvement

There are three Section 4(f) properties along the I-75 corridor: the Withlacoochee State Forest Croom Tract, the Withloacoochee State Trail Park and the Withlacoochee Canoe Trail. In accordance with Section 4(f) of the DOT Act of 1966 (Title 49, U.S.C., Section 1653(f), amended and recodified in Title 49, U.S.C., Section 303, in 1983), the project was examined for possible involvement with Section 4(f) properties.

5.2.2 Archaeological and Historic

A <u>Cultural Resource Assessment Survey Report</u>, January 2006, was prepared for the I-75 PD&E Study. The report included the 98 SMF site alternatives.

The archaeological probability analysis conducted for the project area including the 98 SMF site alternatives concluded that no known sites considered potentially eligible for listing in the National Register of Historic Places (NRHP) are contained within the I-75 PD&E Study project area of potential effect (APE).

The study methodology entailed a review of the available data, including Florida Site File (FSF) records, NRHP listings, relevant cultural resource assessment reports (ACI 1989; Ballo 1988; Deming 1993; 1994a, 1994b; Wharton 1990), US Department of Agriculture (USDA) soil survey maps (Stankey 1982), USGS quadrangle maps, as well as a reconnaissance-level historic structures field survey. Background research indicated an absence of <u>NRHP</u> listed, eligible, or potentially eligible archaeological sites and historic structures within or adjacent to the SMF site alternatives.

The I-75 improvement project will have no involvement with any archaeological sites or historic structures, which are listed, determined eligible, or considered potentially eligible for listing in the <u>NRHP</u>.

5.3 Threatened and Endangered Species

In accordance with Section 7(c) of the Endangered Species act of 1973, as amended, the project corridor was surveyed for the presence of threatened or endangered species. The surveys revealed a pair of nesting Bald Eagles. Area 3a/3bC – is located within the primary zone of this nesting pair of bald eagles. However, Alternative 3a/3bC is a natural depression area and will not require any construction activity, therefore no impact is anticipated. The territory is designated HN-12B with the associated management zones encroaching into the mainline. Impacts within the primary zone will be limited both by the activities involved and time of season. During the construction phase, the *Bald Eagle Monitoring Guidelines* (September 2002 USFWS) will need to be employed to ensure compliance with the Act.

A pair of Florida Sandhill cranes was observed nesting in an herbaceous wetland. The wetland location interfaces with the SMF Alternative 10C. The nesting pair is afforded protection under the Migratory Bird Treaty Act. Coordination with the U.S. Fish and Wildlife Service will need to be initiated if this site is selected, however at this time, this alternative is not the preferred SMF.

5.4 Hazardous Materials and Petroleum Site Data

A Level I Contamination Screening of the I-75 project corridor was conducted to determine the potential for contamination for the SMF site alternatives and FPC sites.

A contamination screening evaluation was prepared pursuant to the FHWA Technical Advisory T 6640.8A, dated October 30, 1987, and in accordance with the FDOT PD&E Manual, Part 2, Chapter 22, dated December 10, 2003. The purpose of the evaluation was to present the preliminary findings of a literature and file review of the potential for finding

hazardous materials and petroleum contamination on parcels along the proposed alignment, which may affect the proposed improvements.

The SMF alternatives have been assigned a hazardous materials potential rating and are summarized in Tables 5 through 36 in Section 8.0. The FDOT hazardous materials rating system was used and include NO, LOW, MEDIUM, and HIGH. All of the alternative SMF sites had a No rating:

6.0 FLOODPLAINS/FLOODWAYS

6.1 Flooding History

FDOT drainage maps, USGS Quadrangle maps, SWFWMD topographic maps, and FEMA FIRMs were used to identify flood-prone areas within the I-75 project corridor. Field inspections were conducted in July 2005 to identify obvious drainage problems. Additionally, people knowledgeable about local drainage conditions (FDOT maintenance personnel) were interviewed in September 2005 and February 2006. This information is provided in Appendix B and summarized as follows: No flooding problems associated with existing drainage conditions have been identified for the length of this project.

6.2 Flood Insurance Rate Maps

FEMA has prepared FIRMs along the I-75 project corridor in Pasco County dated November 18, 1981 and September 30, 1982. FIRMs along I-75 for Hernando County are dated April 17, 1984. FIRMs for the remainder of the project corridor in Sumter County are dated March 15, 1982. These FEMA Flood Maps are shown in Figures 5a through 5f.

6.3 Flood Zone Description

FEMA has designated 100-year base floodplain areas in eight locations along the I-75 project corridor as shown in the FEMA figures below. After further investigation, only three locations resulted in an encroachment to the 100-year floodplain as summarized in Table 3 and five locations did not result in an encroachment. Four of the five areas that did not result in any encroachment into the 100-year floodplain (F-4, F-5, F-6 and F-8) since the I-75 alignment is above the estimated 100-year floodplain elevation. The fifth potential area of encroachment (location F-7) at Sta. 1817+00 (LT) is very minimal, 0.04 acre which is less than 2 percent of the total 100-year floodplain area and therefore not considered. In this case, avoidance measures can be used to eliminate any impacts and/or floodplain compensation can be provided in the preferred SMF for this basin if necessary. Additionally, avoidance measures will be taken in the design phase to minimize any impacts to the 100year floodplain by steepening the side slopes or possibly adding retaining walls in the area of Moody Lake, for example. The areas of encroachment to the 100-year floodplain are designated as Zone A. Zone A is defined as special flood hazard area inundated by 100year flood with no base flood elevations determined. The remainder of the project is designated as Zone X. Zone X is described as areas determined to be outside the 500-year floodplain.













6.4 Floodplain Compensation

The estimated 100-year floodplain elevations were used to determine the estimated floodplain encroachment for floodplain compensation site sizing. The 100-year floodplain delineation and the recommended alignment were delineated on SWFWMD 1" = 200' aerial topographic maps to estimate encroachment volume for the proposed project. The 100-year floodplain elevations and respective estimated floodplain encroachment volumes are summarized in Table 3. The refined encroachment volumes will be determined during the subsequent design phase when more detailed survey and SMF sizing information are available.

Location	Estimated 100-Year Floodplain Elevation (ft)	Estimated Floodplain Encroachment Volume (ac- ft)
F-1	90	0.66
F-2	95	0.18
F-3	106	1.51
	2.35	

Table 3Floodplain Encroachment Summary

6.5 Regulatory Floodways

There are no regulatory floodways within the I-75 project corridor.

7.0 GEOTECHNICAL DATA

A geotechnical review was performed as part of this PD&E Study. The purpose of this review was to obtain preliminary information concerning the general subsurface soil and groundwater conditions along the project alignment and also to characterize the general subsurface stratigraphy, assess the suitability of the project site for the proposed improvements, identify constraints or limitations that the subsurface conditions may impose on the planned construction, and provide preliminary geotechnical recommendations to guide the design and construction of the project. This review included performing a field reconnaissance and a research of existing data and reference materials such as aerial photographs, USDA SCS Soil Survey maps, USGS topographic maps, existing plans, design engineering information for the past construction projects within the study area, and records of sinkhole activity. The findings of this review were presented in a separate document, the <u>Preliminary Geotechnical Report</u>, prepared in August 2005.

To generally assess the near-surface conditions within the limits of the project, the soil maps provided in the "Soil Survey of Pasco, Hernando and Sumter Counties, Florida" were reviewed. The SCS maps are presented on Figures 6a through 6d.

In general, the surficial soils consist of poorly graded fine sands, silty sands and silty to clayey fine sands underlain by clayey fine sands and clays. Some clayey fine sands and clays are encountered at shallow depths of less than 30 inches below the ground surface. Organic soils (muck) may also be encountered in some areas. Seasonal high water levels along I-75 may range from 2.0 feet above the natural ground surface to greater than 6.0 feet below the natural ground surface. Surface and/or subsurface boulders may also be encountered in a few areas near the northern end of the project alignment. The soil groups are summarized in Table 4.

Table 4Summary of Soil Groups

Soil Name	Depth	Class	ification	Permeability	Seasonal High Water	Hydrologic
(Map Unit No.)	(inches)	AASHTO ¹ Group	USCS ² Group	(in/hour)	Table Depth (ft)	Group
Pasco County						
Wachula Fine Sand (1)	0-8 8-19 19-26 26-34 34-80	A-3, A-2-4 SP-SM 6-20 A-3, A-2-4 SP-SM, SM 6-20 A-3, A-2-4 SP-SM, SM 6-20 A-3, A-2-4 SP-SM, SM 0.6-6.0 A-3, A-2-4 SP-SM, SM 0.6-6.0 A-3, A-2-4 SP-SM, SM 0.6-6.0 A-2-4, A-2-6, SM, SM-SC, SC 0.6-6.0		0.0 - 1.0	B/D	
Pomona (2)	0-6 6-22 22-36 36-52 52-60	A-3, A-2-4 A-3, A-2-4 A-3, A-2-4 A-3, A-2-4 A-2-4, A-4, A-6	SP-SM SP, SP-SM, SM SP-SM, SM SP, SP-SM SC-SM-SC, SM	6-20 6-20 0.6-6.0 6-20 0.2-0.6	0 - 1.0	B/D
Tavares Sand (6)	0-86	A-3 SP, SP-SM >20		3.5 – 6.0	А	
Sparr Fine Sand 0 to 5% Slopes (7)	0-6 6-43 43-48 48-59 59-80	A-3, A-2-4 A-3, A-2-4 A-2 A-2, A-4, A-6 A-2, A-4, A-6	SP-SM SP-SM SM-SC, SC, SM SC, SM-SC SC, SM-SC, SM	6-20 6-20 0.6-2.0 0.6-2.0 0.6-2.0	1.5 – 3.5	С
Zephyr Muck (16)	13-0 0-18 18-48 48-67	A-8 A-3, A-2-4 A-2-4, A-2-6 A-2-4, A-4	A-8 PT 6-20 A-3, A-2-4 SP-SM,SM 6-20 A-2-4, A-2-6 SM, SM-SC, SC 0.06-0. A-2-4, A-4 SM, SM-SC, SC 0.6-6.0		+2.0 - 1.0	D
Basinger (23)	0-10 10-30 3-80	A-3 A-3, A-2-4 A-3, A-2-4	SP SP, SP-SM SP, SP-SM	>20 >20 >20	+2.0 – 1.0	B/D
Pits (28)						
Pompano Fine Sand (34)	0-80	A-3, A-2-4	SP, SP-SM	>20	0.0 – 1.0	B/D
Arredondo (44)	0-52 52-55 55-80	A-2-4, A-3 A-2-4 A-2-4, A-2-6 A-4, A-6	SP-SM, SM SM, SM-SC SC, SM-SC	A 6-20 C 2.0-6.0 0 - 1.0 C 0.6-6.0 0 - 1.0		B/D

Soil Name	Depth	Classi	ification	Permeability	Seasonal High Water	Hydrologic
(Map Unit No.)	Jnit No.) (inches) AASHTO ¹ Group USCS ² Group (in/hour)		(in/hour)	Table Depth (ft)	Group	
Lochloosa Fine Sand 0-5% Slopes (48)	0-36 36-42 42-63 63-72 72-80	A-2-4, A-3 A-2-4 A-2, A-6, A-4 A-6, A-7 A-2, A-4, A-6	SP-SM, SM SM, SM-SC SC, SC-SM SC SC, SM-SC	6-20 2.0-6.0 0.6-2.0 0.6-2.0 0.6-2.0	2.5 5.0	С
Blichton Fine Sand 0 to 2% Slopes (49)	0-22 22-28 25-63 63-80	A-2-4, A-3 A-2-4 A-6 A-2-4	SP-SM, SM SM, SM-SC SC SM-SC, SM	6-20 2.0-6.0 0.2-0.6 2.0-6.0	0 – 1.0	D
Blichton Fine Sand, 2 to 5% Slopes (50)	0-38 38-44 44-50 50-62 62-80	A-2-4, A-3 A-2-4 A-6 A-2, A-6, A-7 A-2-4	SP-SM, SM SM, SM-SC SC SM-SC, SM	6-20 2.0-6.0 0.2-0.6 0.2-0.6 2.0-6.0	0 – 1.0	D
Sparr Fine Sand 5 to 8% Slopes (53)	0-6 6-57 57-61 61-69 69-80	A-3, A2-4 A-3, A-2-4 A-2 A-2, A-4, A-6 A-2, A-4, A-6	SP-SM SP-SM SM-SC, SC, SM SC, SM-SC SC, SM-SC, SM	6-20 6-20 0.6-2.0 0.6-2.0 0.6-2.0	1.5 – 3.5	С
Flemington Variant Fine Sand, 2 to 5% Slopes (54)	0-5 5-80	A-3, A-2-4 A-7	SP-SM, SM SC, CL, CH	2.0-20 <0.06	0.0 – 2.5	D
Newnan Fine Sand, 0 to 5% Slopes (59)	0-22 22-33 33-44 44-80	A-3, A-2-4 A-3, A-2-4 A-3, A-2-4 A-2-4, A-4, A-6	A-3, A-2-4 SP, SP-SM 6-20 A-3, A-2-4 SP-SM, SM 2.0-20 A-3, A-2-4 SP, SP-SM, SM 6-20 -2-4, A-4, A-6 SM, SM-SC, SC 0.06-0.6		1.5 – 2.5	С
Palmetto Sellers (60)	0-10 10-46 46-80	A-3, A-2-4 A-3, A-2-4 A-2-4, A-2-6	A-3, A-2-4 SP, SP-SM 6.0-20 A-3, A-2-4 SP-SM 6.0-20 A-2-4, A-2-6 SM, SM-SC, SC 0.2-0.6		+2.0-0	B/D
Hernando Coun	nty					
Arredondo Fine Sand, 0 to 5% Slopes (6)	0-62 62-69 69-99	A-2-4, A-3 A-2-4 A-2-6, A-6	SP-SM, SM SM, SM-SC SC	6-20 2.0-6.0 0.6-6.0	>6.0	А
Arredondo Fine Sand, 5 to 8% Slopes (7)	0-62 62-69 69-99	A-2-4, A-3 SP-SM, SM A-2-4 SM, SM-SC A-2-6, A-6 SC		6-20 2.0-6.0 0.6-6.0	>6.0	A
Basinger (10)	0-80	A-3, A-2-4 SP, SP-SM		>20	+2.0 - 1.0	A/D
Blichton Loamy Fine Sand, 2 to 5% Slopes (12)	0-28 28-34 34-63 63-75	A-2-4, A-3 SP-SM, SM A-2-4, A-6 SC A-6 SC A-6, A-7 SC, CL, CH		6-20 2.0-6.0 0.6-2.0 0.2-0.6	0.0 - 1.0	D
Candler Fine Sand, 0 to 5% Slopes (14)	0-48 48-80	A-3 A-3, A-2-4	SP, SP-SM SP-SM	>20 6-20	>6.0	А

Soil Name	Depth	Classi	fication	Permeability	Seasonal High Water	Hydrologic
(Map Unit No.)	D Unit No.) (inches) AASHTO ¹ Group USCS ² Group (in/hour)		(in/hour)	Table Depth (ft)	Group	
Candler Fine Sand, 5 to 8% Slopes (15)	0-48 48-80	A-3 A-3, A-2-4	SP, SP-SM SP-SM	>20 6-20	>6.0	А
Flemington Fine Sandy Loam, 0 to 2% Slopes (20)	0-5 5-36 36-66 66-81	A-2-4 A-7 A-7 A-7	SM SC, CH, CL CH, MH, CL CH, MH	2.0-20 <0.06 <0.06 <0.06	0.0 – 2.5	D
Floridana- Basinger Association, Occasionally Flooded (24)	Floridana 0-16 16-27 27-80 Bassinge r 0-80	Floridana A-3,A-2-4 A-3 A-2-4, A-2-6 Bassinger A-3, A-2-4	Floridana SP-SM, SM SP, SP-SM SM-SC, SC Bassinger SP, SP-SM	Floridana 6-20 6-20 0.6-2.0 Bassinger >20	0.0 – 1.0	A/D
Kanapaha (28)	0-50 50-56 56-65	A-3, A-2-4 A-2-4 A-2-4, A-2-6, A- 4, A-6	SP-SM SM-SC, SC SC, SM-SC	6.0-20 0.6-2.0 0.2-0.6	0.0 – 1.0	A/D
Kendrick Fine Sand 0 to 5% Slopes (29)	0-28 28-34 34-63 63-80	A-3, A-2-4 A-2-6, A-2-4 A-2-6, A-6 A-2-6, A-2-4	SP-SM SC, SM-SC SC SC, SM-SC	6-20 0.6-2.0 0.6-2.0 0.6-2.0	>6.0	A
Lake Fine Sand 0 to 5% Slopes (31)	0-82	A-3, A-2-4	SP-SM	6-20	>6.0	А
Nobleton fine Sand, 0 to 5% Slopes (36)	0-33 33-37 37-60 60-80 80-85	A-2-4 A-2-6,A-6 A-6, A-7 A-2-6,A-6 A-2-4,A-2-6 A-6	SP-SM, SM SC SC, CL, CH SC SM, SM-SC, SC	6-20 0.2-2.0 0.2-0.6 0.2-2.0 0.2-6.0	1.5 – 3.5	A
Pits (41)						
Sparr Fine Sand 0 to 5% Slopes (47)	0-61 61-64 64-80	A-3, A-2-4 A-2-4 A-2-4, A-2-6 A-4, A-6	SP-SM SM-SC, SM SC, SM-SC	6-20 0.6-2.0 0.6-2.0	1.5 – 3.5	A
Sparr Fine Sand 5 to 8% Slopes (48)	0-61 61-64 64-80	A-3, A-2-4 A-2-4 A-2-4, A-2-6 A-4, A-6	SP-SM SM-SC, SM SC, SM-SC	6-20 0.6-2.0 0.6-2.0	1.5 – 3.5	A
Tavares (49)	0-80	A-3 SP, SP-SM >20		>20	3.5 – 6.0	A

Soil Name	Depth	Class	ification	Permeability	Seasonal High Water	Hydrologic
(Map Unit No.)	(inches)	AASHTO ¹ Group USCS ² Group		(in/hour)	Table Depth (ft)	Group
Wauchula (52)	0-8 8-24 24-31 31-38 38-80	A-3, A-2-4 A-3, A-2-4 A-3, A-2-4 A-3, A-2-4 A-2-4, A-2-6, A- 4, A-6	A-3, A-2-4 SP-SM 6-20 A-3, A-2-4 SP-SM 6-20 A-3, A-2-4 SP-SM, SM 0.6-6.0 A-3, A-2-4 SP-SM, SM 6-20 A-3, A-2-4 SP-SM, SM 0.6-6.0 A-3, A-2-4 SP-SM, SM 6-20 A-2-4, A-2-6, A- SM, SM-SC, SC 0.6-6.0 4, A-6 A-2 A-2		0.0 – 1.0	B/D
Sumter County						
Candler Fine Sand, 0 to 5% Slopes (4)	0-8 8-50 50-80	A-3 A-3 A-3, A-2-4	SP, SP-SM SP, SP-SM SP-SM	6-20 6-20 6-20	>6.0	А
Candler Fine Sand, 5 to 8% Slopes (5)	0-6 6-56 56-80	A-3 A-3 A-3, A-2-4	SP, SP-SM SP, SP-SM SP-SM	6-20	>6.0	А
Lake (8)	0-80	A-3, A-2-4 SP-SM		>6.0	>6.0	А
Adamsville (15)	0-5 5-80	A-3, A-2-4 A-3, A-2-4	A-3, A-2-4 SP-SM A-3, A-2-4 SP-SM, SM		2.0 - 3.5	С
EauGallie Fine Sand, Bouldery Subsurface (21)	0-8 8-25 25-36 36-57 57-80	A-3 A-3 A-3, A-2-4 A-3, A-2-4 A-2-4, A-2-6	SP SP SP-SM,SM SP, SP-SM SM, SM-SC, SC	6-20 6-20 0.6-6 6-20 0.2-0.6	0 – 1.0	B/D
Sumterville Fine Sand, Bouldery Subsurface, 0 to 5% Slopes (27)	0-9 9-29 29-80	A-3, A-2-4 A-3, A-2-4 A-7	SP-SM, SM SP-SM, SM CL, CH	6-20 6-20 0.06-0.2	1.5 – 3.0	С
Nitaw Muck, Frequently Flooded (29)	0-5 5-12 12-65 65-80	A-8 A-3, A-2-4 A-7 A-3, A-2-4	PT SP-SM, SM CH, CL SP, SP-SM,SM SM-SC	6-20 6-20 0.06-0.2 6-20	0 - 1.0	D
Pits (51)						

Source: Soil Survey of Pasco County, June 1982, Soil Survey of Hernando County, July 1977 and Soil Survey of Sumter County, April, 1984.

Notes: ¹American Association of State Highway and Transportation Officials ²Unified Soil Classification System

³SP - Poorly graded sand (with gravel)
 ⁴SP-SM - Poorly graded sand (with sand and gravel)
 ⁵SM - Silty sand (with gravel)

A copy of the soil survey map for the I-75 project corridor is shown in Figures 6.



Figure 6a



From N. of SR 52 to S. of CR 476B Pasco, Hernando & Sumter Counties WPI Seg. No.: 411014 1 FAP No.: 0751- 120I

Map Unit Soil Name Seasonal High Water Table Depth (ID) Hydrologic Group 6 Arredondo Fine Sand 5 to 8% Slopes >6.0 A 7 Arredondo Fine Sand 5 to 8% Slopes >6.0 A 10 Basinger +2.0 - 1.0 A/D 11 Bilchon Loamy Fine Sand, 2 to 5% Slopes 0.0 - 1.0 D 12 Bilchon Loamy Fine Sand, 2 to 5% Slopes >6.0 A 13 Candler Fine Sand 5 to 8% Slopes >6.0 A 20 Flemington Fine Sandy Loam, 0 to 5% Slopes >6.0 A 20 Flemington Fine Sandy Loam, 0 to 5% Slopes >6.0 A 24 Fasciation, Occasionally Flooded >0.0 - 1.0 D/D 28 Kanapaha +2.0 - 1.0 D 29 Kendrick Fine Sand 0 to 5% Slopes >6.0 A 31 0 to 5% Slopes >6.0 A 34 Nobleton fine Sand, 0 to 5% Slopes 1.5 to 3.5 A 41 Pits - 48 Sparr Fine Sand 1.5 to 3.5 A </th <th>N.S.</th> <th colspan="5">HERNANDO COUNTY</th>	N.S.	HERNANDO COUNTY				
6 Arredondo Fine Sand 5 to 8% Stopes >6.0 A 7 Arredondo Fine Sand 5 to 8% Stopes >6.0 A 10 Basinger +2.0 - 1.0 A/D 11 Blichton Learny Fine Sand, 2 to 5% Stopes 0.0 - 1.0 D 12 Blichton Learny Fine Sand, 2 to 5% Stopes >6.0 A 13 Candler Fine Sand 0 to 5% Stopes >6.0 A 14 Candler Fine Sand 5 to 8% Stopes >6.0 A 20 Flemington Fine Sand 5 to 8% Stopes 0.0 - 2.5 D 24 Floridana-Basinger Association, Occasionally Flooded 0.0 - 1.0 A/D 28 Kanapaha +2.0 - 1.0 D 29 Vicame Sand, 0 to 5% Stopes >6.0 A 31 Lake Fine Sand 0 to 5% Stopes >6.0 A 34 Nobleton fine Sand, 0 to 5% Stopes 1.5 to 3.5 A 41 Pits - 47 Sparr Fine Sand 5 to 8% Stopes 1.5 to 3.5 A 48 Sparr Fine Sand 5 to 8% Stopes 1.5 t	28	Map Unit No.	Soil Name	Seasonal High Water Table Depth (ft)	Hydrologic Group	
7 Arredondo Fine Sand 5 to 8% Slopes >6.0 A 10 Basinger +2.0 - 1.0 A/D 12 Blichton Loamy Fine Sand, 2 to 5% Slopes >6.0 A 12 Blichton Loamy Fine Sand, 2 to 5% Slopes >6.0 A 13 Candler Fine Sand 0 to 5% Slopes >6.0 A 14 Oralder Fine Sand 5 to 8% Slopes >6.0 A 20 Flemington Fine Sand 5 to 8% Slopes 0.0 - 1.0 A/D 24 Floridana-Basinger Association, Occasionally Flooded 0.0 - 1.0 A/D 28 Kanapaha +2.0 - 1.0 D 2 29 Kendrick Fine Sand 0 to 5% Slopes >6.0 A 31 Lake Fine Sand 0 to 5% Slopes 1.5 to 3.5 A 41 Pits - - 47 Sparr Fine Sand 5 to 8% Slopes 1.5 to 3.5 A 48 Sparr Fine Sand 5 to 8% Slopes 1.5 to 3.5 A 49 Tavares 3.5 - 6.0 A 50 % Slopes 1.5 to 3.5 <td< td=""><th>Saint</th><td>6</td><td>Arredondo Fine Sand 0 to 5% Slopes</td><td>>6.0</td><td>A</td></td<>	Saint	6	Arredondo Fine Sand 0 to 5% Slopes	>6.0	A	
10 Basinger +2.0 - 1.0 A/D 12 Bilchton Loamy Fine Sand, 2 to 5% Stopes 0.0 - 1.0 D 14 Candler Fine Sand 5 to 8% Stopes >6.0 A 15 Candler Fine Sand 5 to 8% Stopes >6.0 A 20 Flemington Fine Sandy Loam, 0 to 2% Stopes 0.0 - 2.5 D 24 Floridana-Basinger Association, Occasionally Flooded 0.0 - 1.0 A/D 22 Flemington Fine Sandy Loam, 0 to 2% Stopes >6.0 A 31 Lake Fine Sand 0 to 5% Stopes >6.0 A 31 Lake Fine Sand, 0 to 5% Stopes >6.0 A 36 Nobleton fine Sand, 0 to 5% Stopes 1.5 to 3.5 A 41 Pts - - 47 Sparr Fine Sand 0 to 5% Stopes 1.5 to 3.5 A 48 Sparr Fine Sand 5 to 8% Stopes 1.5 to 3.5 A 49 Tavares 3.5 - 6.0 A 52 Wauchula 0 - 1.0 B/D 50 A 5 A 5	Cluir I	7	Arredondo Fine Sand 5 to 8% Slopes	>6.0	А	
12 Blichton Loamy Fine Sand, 2 to 5% Slopes 0.0 - 1.0 D 14 Candler Fine Sand 0 to 5% Slopes >6.0 A 15 Candler Fine Sand 5 to 8% Slopes >6.0 A 20 Flemington Fine Sandy Loam, 0 to 2% Slopes 0.0 - 2.5 D 24 Association, Association, Occasionally Flooded 0.0 - 1.0 A/D 28 Kanapaha +2.0 - 1.0 D 29 Kendrick Fine Sand 0 to 5% Slopes >6.0 A 31 Lake Fine Sand 0 to 5% Slopes >6.0 A 31 D to 5% Slopes >6.0 A 31 O to 5% Slopes >6.0 A 31 O to 5% Slopes 1.5 to 3.5 A 41 Pits - - 47 Sparr Fine Sand 0 to 5% Slopes 1.5 to 3.5 A 48 Sparr Fine Sand 0 to 5% Slopes 1.5 to 3.5 A 49 Tavares 3.5 - 6.0 A 50 Stational Stati	Jake 33	10	Basinger	+2.0 – 1.0	A/D	
14 Candler Fine Sand 0 to 5% Stopes >6.0 A 15 Candler Fine Sand 5 to 8% Stopes >6.0 A 20 Flemington Fine Sandy Loam, 0 to 2% Stopes 0.0 - 2.5 D 24 Association, Occasionally Flooded 0.0 - 1.0 A/D 28 Kanapaha +2.0 - 1.0 D 29 Kendrick Fine Sand 0 to 5% Stopes >6.0 A 31 Lake Fine Sand 0 to 5% Stopes >6.0 A 36 Nobleton fine Sand, 0 to 5% Stopes 1.5 to 3.5 A 41 Pits 47 Sparr Fine Sand 0 to 5% Stopes 1.5 to 3.5 A 48 Sparr Fine Sand 5 to 8% Stopes 1.5 to 3.5 A 49 Tavares 3.5 - 6.0 A 52 Wauchula 0 - 1.0 B/D 50 - - - - 50 - - - - - 50 - - - - - - <td< td=""><th>Real L</th><td>12</td><td>Blichton Loamy Fine Sand, 2 to 5% Slopes</td><td>0.0 - 1.0</td><td>D</td></td<>	Real L	12	Blichton Loamy Fine Sand, 2 to 5% Slopes	0.0 - 1.0	D	
15 Candler Fine Sand 5 to 8% Stopes >6.0 A 20 Flemington Fine Sandy Loam, 0 to 2% Stopes 0.0 – 2.5 D 24 Floridana-Basinger Association, Occasionally Flooded 0.0 – 1.0 A/D 28 Kanapaha +2.0 – 1.0 D 29 Kendrick Fine Sand 0 to 5% Stopes >6.0 A 31 0 to 5% Stopes >6.0 A 36 Nobleton fine Sand, 0 to 5% Stopes 1.5 to 3.5 A 41 Pits 47 Sparr Fine Sand 0 to 5% Stopes 1.5 to 3.5 A 48 Sparr Fine Sand 0 to 5% Stopes 1.5 to 3.5 A 49 Tavares 3.5 – 6.0 A 50 Wauchula 0 – 1.0 B/D	47	14	Candler Fine Sand 0 to 5% Slopes	>6.0	A	
20 Flemington Fine Sandy Loam, 0 to 2% Slopes 0.0 – 2.5 D 24 Association, Occasionally Flooded 0.0 – 1.0 A/D 28 Kanapaha +2.0 – 1.0 D 29 Kendrick Fine Sand 0 to 5% Slopes >6.0 A 31 Lake Fine Sand 0 to 5% Slopes >6.0 A 36 Nobleton fine Sand, 0 to 5% Slopes 1.5 to 3.5 A 41 Pits 47 Sparr Fine Sand 5 to 8% Slopes 1.5 to 3.5 A 48 Sparr Fine Sand 5 to 8% Slopes 1.5 to 3.5 A 49 Tavares 3.5 - 6.0 A 49 Tavares 3.5 - 6.0 A 50 Wauchula 0 - 1.0 B/D 50 Wauchula 0 - 1.0 B/D	48	15	Candler Fine Sand 5 to 8% Slopes	>6.0	A	
24 Floridana-Basinger Association, Occasionally Flooded 0.0 – 1.0 A/D 28 Kanapaha +2.0 – 1.0 D 29 Kendrick Fine Sand 0 to 5% Slopes >6.0 A 31 Lake Fine Sand 0 to 5% Slopes >6.0 A 36 Nobleton fine Sand, 0 to 5% Slopes 1.5 to 3.5 A 41 Pits - - 47 Sparr Fine Sand 0 to 5% Slopes 1.5 to 3.5 A 48 Sparr Fine Sand 5 to 8% Slopes 1.5 to 3.5 A 49 Tavares 3.5 - 6.0 A 50 A 5 to 8% Slopes 1.5 to 3.5 A 50 A 9 Tavares 3.5 - 6.0 A 50 A 9 Tavares 3.5 - 6.0 A 50 A 9 Tavares 3.5 - 6.0 A 50 A 9 A A A A 60 A 9 A A A A A </th <th>541</th> <th>20</th> <th>Flemington Fine Sandy Loam, 0 to 2% Slopes</th> <th>0.0 – 2.5</th> <th>D</th>	541	20	Flemington Fine Sandy Loam, 0 to 2% Slopes	0.0 – 2.5	D	
28 Kanapaha +2.0 - 1.0 D 29 Kendrick Fine Sand 0 to 5% Slopes >6.0 A 31 Lake Fine Sand 0 to 5% Slopes >6.0 A 36 Nobleton fine Sand, 0 to 5% Slopes 1.5 to 3.5 A 41 Pits 47 Sparr Fine Sand 0 to 5% Slopes 1.5 to 3.5 A 48 Sparr Fine Sand 5 to 8% Slopes 1.5 to 3.5 A 49 Tavares 3.5 - 6.0 A 52 Wauchula 0 - 1.0 B/D 50 % % % % 50 % % % % 60 % % % % 9 Tavares 3.5 - 6.0 A 52 Wauchula 0 - 1.0 B/D 7 % % % % 80 % % % % %	$)^{\circ})$	24	Floridana-Basinger Association, Occasionally Flooded	0.0 – 1.0	A/D	
29 Kendrick Fine Sand 0 to 5% Slopes >6.0 A 31 Lake Fine Sand 0 to 5% Slopes >6.0 A 36 Nobleton fine Sand, 0 to 5% Slopes 1.5 to 3.5 A 41 Pits 47 Sparr Fine Sand 0 to 5% Slopes 1.5 to 3.5 A 48 Sparr Fine Sand 5 to 8% Slopes 1.5 to 3.5 A 49 Tavares 3.5 - 6.0 A 52 Wauchula 0 - 1.0 B/D 40 Tavares 3.5 - 6.0 A 52 Wauchula 0 - 1.0 B/D 49 Tavares 3.5 - 6.0 A 52 Wauchula 0 - 1.0 B/D 49 Tavares 3.5 - 6.0 A 50 0 0 0 0 4 50 0 0 0 4 0	2	28	Kanapaha	+2.0 - 1.0	D	
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36 Nobleton fine Sand, 0 to 5% Slopes 1.5 to 3.5 A 41 Pits 47 Sparr Fine Sand 0 to 5% Slopes 1.5 to 3.5 A 48 Sparr Fine Sand 5 to 8% Slopes 1.5 to 3.5 A 49 Tavares 3.5 - 6.0 A 52 Wauchula 0 - 1.0 B/D 40 Fine Sand 5 to 8% Slopes 1.5 to 3.5 A 49 Tavares 3.5 - 6.0 A 52 Wauchula 0 - 1.0 B/D 40 Fine Sand 5 to 8% Slopes 1.5 to 3.5 A 50 A 9 Tavares 3.5 - 6.0 A 52 Wauchula 0 - 1.0 B/D A A 51 A A A A A A 51 A A A A A A	A	31	Lake Fine Sand 0 to 5% Slopes	>6.0	А	
41 Pits 47 Sparr Fine Sand 0 to 5% Slopes 1.5 to 3.5 A 48 Sparr Fine Sand 5 to 8% Slopes 1.5 to 3.5 A 49 Tavares 3.5 - 6.0 A 52 Wauchula 0 - 1.0 B/D 40 File 0 0 0 49 Tavares 3.5 - 6.0 A 52 52 Wauchula 0 - 1.0 B/D 0 49 Tavares 3.5 - 6.0 A 52 52 Wauchula 0 - 1.0 B/D 0 49 Tavares 3.5 - 6.0 A 52 52 Wauchula 0 - 1.0 B/D 0 0 40 Tavares 3.5 - 6.0 A 50 0	3	36	Nobleton fine Sand, 0 to 5% Slopes	1.5 to 3.5	А	
47 Sparr Fine Sand 0 to 5% Slopes 1.5 to 3.5 A 48 Sparr Fine Sand 5 to 8% Slopes 1.5 to 3.5 A 49 Tavares 3.5 - 6.0 A 52 Wauchula 0 - 1.0 B/D 40 Tavares 3.5 - 6.0 A 52 Wauchula 0 - 1.0 B/D 49 Tavares 0.0 0.0 40 Tavares Tavares Tavares 40 Tavares Tavares Tavares 40 Tavar	4	41	Pits			
48 Sparr Fine Sand 5 to 8% Slopes 1.5 to 3.5 A 49 Tavares 3.5 - 6.0 A 52 Wauchula 0 - 1.0 B/D 49 Tavares 3.5 - 6.0 A 52 Wauchula 0 - 1.0 B/D 49 Tavares 3.5 - 6.0 A 52 Wauchula 0 - 1.0 B/D 49 Tavares 3.5 - 6.0 A 50 4 9 Tavares 3.5 - 6.0 50 4 9 Tavares 3.5 - 6.0 50 4 9 7 9 50 4 9 7 9 50 4 9 7 9 50 4 9 9 9 9 6 9 9 9 9 9 7 9 9 9 9 9 7 9 9 9 9 9 <	25	47	Sparr Fine Sand 0 to 5% Slopes	1.5 to 3.5	A	
49 Tavares 3.5 - 6.0 A 52 Wauchula 0 - 1.0 B/D 49 6 0	28	48	Sparr Fine Sand 5 to 8% Slopes	1.5 to 3.5	А	
52 Wauchula 0-1.0 B/D		49	Tavares	3.5 – 6.0	A	
4 4 4 4 4 4 4 4 4 4	41	52	Wauchula	0 – 1.0	B/D	
	$\begin{array}{c} 439 \\ 6 \\ 78 \\ 78 \\ 78 \\ 78 \\ 78 \\ 78 \\ 78 $			Robison 28 Lake 12 47 47 29		

Soils Map

Figure 6b



From N. of SR 52 to S. of CR 476B Pasco, Hernando & Sumter Counties WPI Seg. No.: 411014 1 FAP No.: 0751- 120I

W	HERNANDO COUNTY					
	Map Unit No.	Soil Name	Seasonal High Water Table Depth (ft)	Hydrologic Group		
	6	Arredondo Fine Sand 0 to 5% Slopes	>6.0	A		
V H	7	Arredondo Fine Sand 5 to 8% Slopes	>6.0	A		
N.T.S.	10	Basinger	+2.0 – 1.0	A/D		
0	12	Blichton Loamy Fine Sand, 2 to 5% Slopes	0.0 – 1.0	D		
(C	14	Candler Fine Sand 0 to 5% Slopes	>6.0	A		
50	15	Candler Fine Sand 5 to 8% Slopes	>6.0	А		
	20	Flemington Fine Sandy Loam, 0 to 2% Slopes	0.0 – 2.5	D		
198	24	Floridana-Basinger Association, Occasionally Flooded	0.0 – 1.0	A/D		
	28	Kanapaha	+2.0 – 1.0	D		
	29	Kendrick Fine Sand 0 to 5% Slopes	>6.0	А		
)	31	Lake Fine Sand 0 to 5% Slopes	>6.0	А		
1s_	36	Nobleton fine Sand, 0 to 5% Slopes	1.5 to 3.5	А		
	41	Pits				
	47	Sparr Fine Sand 0 to 5% Slopes	1.5 to 3.5	А		
	48	Sparr Fine Sand 5 to 8% Slopes	1.5 to 3.5	А		
5	49	Tavares	3.5 – 6.0	А		
14	52	Wauchula	0 – 1.0	B/D		
		- /	11	50 4 F 63		

Soils Map

Figure 6c
6	OREST	31	U	T	5			And the second s
	~ 1		15	HE	RNAND		NTY.	
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	Δ	- 15 -		2			$\sum_{i=1}^{n}$	
(75)	t	33	14		2 15 (B) 21 (B) 41		J.	
		E	P	A de la	20 20	NTHLACOOCHE	o (
- 1	HERNANDO		10		ne the	E	1	A and a super S
Map Unit No.	Soil Name	Seasonal High Water Table Depth (ft)	Hydrologic Group	C		STATE	41 A1	ad () and ochee Bar a
6	Arredondo Fine Sand	>6.0	A) ° crc	om Rit	alto	E shlacoe
7	Arredondo Fine Sand	>6.0	A	19 IA	очновная	0	A	
10	Basinger	+2.0 - 1.0	A/D	+	5-5-4		47	
12	Blichton Loamy Fine	0.0 – 1.0	D		· ~ A	TEST	\sim	
14	Candler Fine Sand	>6.0	A	5	Carl -	14		
15	Candler Fine Sand	>6.0	A	Rock	Provide States	V J	Car	
20	Flemington Fine Sandy	0.0 – 2.5	D	49	47		2 25	
24	Loam, 0 to 2% Slopes Floridana-Basinger Association, Occasionally Elooded	0.0 - 1.0	A/D	Map Unit	Soil Name	Seasonal High Water Table	Hydrologic	July Han Start And Start
28	Kanapaha	+2.0 - 1.0	D	No.	Candler Fine Sand. 0	Depth (ft)	Cicup	AG WILL CH T
29	Kendrick Fine Sand 0 to 5% Slopes	>6.0	А	4	to 5% Slopes	>0.0	A	29 29 29
31	Lake Fine Sand 0 to 5% Slopes	>6.0	А	5	to 8% Slopes	>6.0	A	The second secon
36	Nobleton fine Sand, 0 to 5% Slopes	1.5 to 3.5	A	8	Lake	>6.0	A	
41	Pits			15	Adamsville	2.0 - 3.5	С	
47	Sparr Fine Sand	1.5 to 3.5	А	21	Bouldery Subsurface	0 – 1.0	B/D	Phan Co
48	Sparr Fine Sand 5 to 8% Slopes	1.5 to 3.5	A	27	Sumterville Fine Sand, Bouldery Subsurface, 0 to 5% Slopes	1.5 – 3.0	с	a the and the solution
49	Tavares	3.5 - 6.0	A	29	Nitaw Muck, Frequently Flooded	0 – 1.0	D	Soil Survey of Hernando County, Florida, July 1977.
52	Wauchula	0 – 1.0	B/D	51	Pits			and <u>Soil Survey of Sumter County</u> , Florida, 1988.
SINTE O	1-75	PD&E	Stuc	dy				

From N. of SR 52 to S. of CR 476B Pasco, Hernando & Sumter Counties

WPI Seg. No.: 411014 1 FAP No.: 0751- 1201



8.0 ALTERNATIVE SMF SITE INFORMATION

8.1 Stormwater Management Methodology and Criteria

A review of the best available information listed in Section 1.0 of this report in addition to field reconnaissance was conducted to assess the potential SMF locations. The following parameters of each site were analyzed in the selection process:

- The "Available Area" for each alternative was obtained from the Pasco, Hernando and Sumter Counties Property Appraiser's Tax Maps.
- The "Existing Average Ground Elevation" was obtained from the SWFWMD Aerials (1"=200'), as shown in Appendix D.
- The "Soil Type" information was obtained for each of the alternatives from the SCS Soil Survey for Pasco, Hernando and Sumter Counties, Florida. The seasonal high water table (SHWT) elevation was estimated by subtracting the average depth to the SHWT from the average existing ground elevation.
- The maximum stage in the SMF for a 100-year storm event (DHW₁₀₀) was estimated using the following procedure. Pre and post CN numbers were calculated along with an estimated time of concentration (Tc) which was then used to determine the 25year and 100-year peak outflow using TR55 (see "Q" – Post Development Conditions (8-Lanes) table in Appendix C). The estimated DHW was used in the "Estimated Stormwater Management Facility (SMF) Area Requirements (8-Lanes)" table in Appendix C to estimate the SMF sizes.
- The "Impact on Wetlands, Cultural Resources, Threatened or Endangered Species" and "Contamination Impact" is based on the information included in Section 5.0 of this report.
- The "Right-of-Way Cost Estimate" information was approved by the FDOT Right-of-Way Department.

8.2 SMF Alternative Analysis

The project has been divided into 32 roadway drainage basins, as shown in the Drainage Basin Map in Figure 3. Basin 1 was not included due to a change in the project limits.

Based on the methodology and criteria stated in Section 8.1, the following alternative SMF sites were evaluated for each basin. SMF site alternatives are labeled SMF Site 2A, for example.

- 1) Basin 2: SMF Sites 2A, 2B and 2C
- 2) Basin 3: SMF Sites 3A, 3B and 3C
- 3) Basin 4: SMF Sites 4A, 4B and 4C
- 4) Basin 5: SMF Sites 5A, 5B and 5C
- 5) Basin 6: SMF Sites 6A, 6B and 6C

- 6) Basin 7: SMF Sites 7A, 7B and 7C
- 7) Basin 8: SMF Sites 8A, 8B and 8C
- 8) Basin 9: SMF Sites 9A, 9B and 9C
- 9) Basin 10: SMF Sites 10A, 10B and 10C
- 10) Basin 11: SMF Sites 11A and 11B
- 11) Basin 12: SMF Sites 12A and 12B
- 12) Basin 13: SMF Sites 13A, 13B and 13C
- 13) Basin 14: SMF Sites 14A, 14B and 14C
- 14) Basin 15: SMF Sites 15A, 15B and 15C
- 15) Basin 16: SMF Sites 16A and 16B
- 16) Basin 17: SMF Sites 17A and 17B
- 17) Basin 18: SMF Sites 18A and 18B
- 18) Basin 19: SMF Sites 19A, 19B and 19C
- 19) Basin 20: SMF Sites 20A, 20B and 20C
- 20) Basin 21: SMF Sites 21A, 21B and 21C
- 21) Basin 22: SMF Sites 22A, 22B and 22C
- 22) Basin 23: SMF Sites 23A, 23B and 23C
- 23) Basin 24: SMF Sites 24A, 24B and 24C
- 24) Basin 25: SMF Sites 25A, 25B and 25C
- 25) Basin 26: SMF Sites 26A, 26B and 26C
- 26) Basin 27: SMF Sites 27A, 27B and 27C
- 27) Basin 29: SMF Sites 29A, 29B and 29C
- 28) Basin 30: SMF 30A; SMF 30B; SMF 30C & 3a/3Bc; and SMF 30D & 3a/3bC
- 29) Basin 31: SMF 31A; SMF 31B; 4a(e)C, 4a(w)C, 4b(e)C & 4b(w)C; and SMF 31D, 4b(e)C & 4b(w)C
- 30) Basin 32: SMF 32A, SMF 32B and 5aC & 5bC
- 31) Basin 33: SMF 33A, SMF 33B and 6a/bC & 6cC

32) Basin 34: SMF 34A, 34B and 7C

Each alternative is summarized in the SMF Alternative Matrix Analyses in Tables 5 through 36. The locations of the alternative SMF sites are shown on the Concept Plans in Appendix A. The SMFs are sized to accommodate the required treatment and attenuation per basin. The treatment volume was calculated for 1 inch over the directly connected impervious area (DCIA). Attenuation volumes were calculated using the SCS 100-year/24-hour post minus pre volumes per basin. Weighted curve numbers (CNs) were calculated using the proposed minimum right-of-way width of 150 feet. The calculations used to estimate the size of the SMFs are included in Appendix C.

The resulting information in Tables 5 through 36 was used to analyze each SMF site for selection of a preferred alternative for each basin. The preferred alternatives per basin are highlighted in the tables and the recommendations are summarized in Section 9.0.

The information in Tables 37 through 40 determine a preferred location for floodplain compensation based on the estimated impact to the 100-year floodplain for the right-of-way as well as any impacts to the preferred SMF. The potential locations identified were either a portion of an SMF site that was not the preferred or adjacent in the preferred SMF site. Other factors, in addition to the environmental criteria, that were used to select the preferred FPC Site include location to the estimated elevation of the 100-year floodplain elevation and the soil type. A proportion of the total right-of-way cost was used to estimate the right-of-way cost for the area of floodplain compensation required. For example, if the estimated right-of-way cost for a SMF Alternative was \$500,000 and the estimated area needed for floodplain compensation was 50% of that site, then the estimated right-of-way cost would be \$250,000.

	SMF Site Alternatives		
Alternative	2A	2B	2C
Location (Station) / Side (LT, RT)	1255+00/RT	1259+00/LT	1259+00/RT
SMF Area (acres)	2.7	2.7	2.8
Soils Names & Hydrologic Groups	Pomona (B/D) & Palmetto Sellers (B/D)	Pomona (B/D) & Pits	Pomona (B/D) & Pits
Proximity to Outfall (feet)	0	0	0
Pipe Costs (Assume 36" Class II Conc. Pipe @ \$47/LF)	\$12,455	\$O	\$8,695
SMF Impacts to the 100-YR Floodplain (ac)	0	0	0
Right-of-Way Cost Estimate for 100-YR Floodplain Impact to SMF	\$0	\$0	\$0
Recorded Archaeological Sites	None	None	N/A
Impact to Section 4(f) Property	No	No	No
Wetlands (acres)	0.34	0.25	0.07
Wetland Mitigation Cost (\$95,000/acre)	\$32,3000	\$23,750	\$6,650
Threatened and Endangered Species (Plant and Animals)	None	None	None
Contamination Risk Rating	No	No	No
Right-of-Way Cost Estimate	\$459,600	\$529,800	\$593,500
Total Estimated Cost	\$504,355	\$553,550	\$608,845

Table 5 SMF Alternative Matrix Analyses Basin 2

Notes:

• No floodplain compensation is required for Basin 2.

Table 6						
SMF Alternative Matrix Analyses						
Basin 3						

	S	MF Site Alternatives	
Alternative	3A	3B	3C
Location (Station) / Side (LT, RT)	1284+00/LT	1287+00/RT	1281+00/RT
SMF Area (acres)	2.2	2.2	2.2
Soils Names & Hydrologic Groups	Lochloosa Fine Sand (C)	Pomona (B/D)	Pomona (B/D)
Proximity to Outfall (feet)	0	0	0
Pipe Costs (Assume 36" Class II Conc. Pipe @ \$47/LF)	\$0	\$0	\$0
SMF Impacts to the 100-YR Floodplain (ac)	0	2.18	0
Right-of-Way Cost Estimate for 100-YR Floodplain Impact to SMF	\$0	\$395,100	\$0
Recorded Archaeological Sites	None	None	None
Impact to Section 4(f) Property	No	No	No
Wetlands (acres)	0	0	0
Wetland Mitigation Cost (\$95,000/acre)	\$0	\$0	\$0
Threatened and Endangered Species (Plant and Animals)	None	None	None
Contamination Risk Rating	No	No	No
SMF Right-of-Way Cost Estimate	\$449,500	\$395,100	\$392,700
Total Estimated Cost	\$449,500	\$790,200	\$392,700

Notes:

• Basin 3 requires floodplain compensation. See Table 37 for floodplain compensation.

	S	MF Site Alternativ	/es
Alternative	4A	4B	4C
Location (Station) / Side (LT, RT)	1303+00/RT	1298+00/RT	1301+00/LT
SMF Area (acres)	4.0	4.1	4.0
Soils Names & Hydrologic Groups	Newnan Fine Sand (C)	Newnan Fine Sand (C)	Newnan Fine Sand (C)
Proximity to Outfall (feet)	0	0	0
Pipe Costs (Assume 36" Class II Conc. Pipe @ \$47/LF)	\$0	\$0	\$0
SMF Impacts to the 100-YR Floodplain (ac)	0	1.97	3.42
Right-of-Way Cost Estimate for 100-YR Floodplain Impact to SMF	\$O	\$309,430	\$470,950
Recorded Archaeological Sites	None	None	None
Impact to Section 4(f) Property	No	No	No
Wetlands	0	0	0.58
Wetland Mitigation Cost (\$95,000/acre)	\$0	\$0	\$55,100
Threatened and Endangered Species (Plant and Animals)	None	None	None
Contamination Risk Rating	No	No	No
Right-of-Way Cost Estimate	\$1,575,000	\$648,700	\$552,200
Total Estimated Cost	\$1,575,000	\$958,130	\$1,078,250

Table 7 SMF Alternative Matrix Analyses Basin 4

Notes:

• Basin 4 requires floodplain compensation. See Table 37 for floodplain compensation.

	S	MF Site Alternativ	/es
Alternative	5A	5B	5C
Location (Station) / Side (LT,RT)	1342+00/LT	1336+00/RT	1339+00/RT
SMF Area (acres)	5.0	4.6	4.3
Soils Names & Hydrologic Groups	Newnan Fine Sand (C)	Newnan Fine Sand (C)	Newnan Fine Sand (C)
Proximity to Outfall (feet)	0	0	0
Pipe Costs (Assume 36" Class II Conc. Pipe @ \$47/LF)	\$0	\$0	\$0
SMF Impacts to the 100-YR Floodplain (ac)	0	0.14	0.20
Right-of-Way Cost Estimate for 100-YR Floodplain Impact to SMF	\$0	\$21,640	\$31,630
Recorded Archaeological Sites	None	None	None
Impact to Section 4(f) Property	No	No	No
Wetlands	0.12	0.18	0.18
Wetland Mitigation Cost (\$95,000/acre)	\$11,400	\$17,100	\$17,100
Threatened and Endangered Species (Plant and Animals)	None	None	None
Contamination Risk Rating	No	No	No
Right-of-Way Cost Estimate	\$1,106,300	\$686,300	\$643,600
Total Estimated Cost	\$1,117,700	\$725,040	\$692,330

Table 8 SMF Alternative Matrix Analyses Basin 5

Notes:

• No floodplain compensation is required for Basin 5.

	SN	IF Site Alternative	s
Alternative	6A	6B	6C
Location (Station) / Side (LT, RT)	1375+00/LT	1383+00/LT	1380+00/LT
SMF Area (acres)	3.1	3.1	3.1
Soils Names & Hydrologic Groups	Pomona (B/D) & Blichton Fine Sand (D)	Pomona (B/D) & Blichton Fine Sand (D)	Pomona (B/D) & Blichton Fine Sand (D)
Proximity to Outfall (feet)	0	0	0
Pipe Costs (Assume 36" Class II Conc. Pipe @ \$47/LF)	\$0	\$0	\$0
SMF Impacts to the 100-YR Floodplain (ac)	0	0	0
Right-of-Way Cost Estimate for 100- YR Floodplain Impact to SMF	\$0	\$O	\$0
Recorded Archaeological Sites	None	None	None
Impact to Section 4(f) Property	No	No	No
Wetlands	0.12	0	0
Wetland Mitigation Cost (\$95,000/acre)	\$11,400	\$0	\$0
Threatened and Endangered Species (Plant and Animals)	None	None	None
Contamination Risk Rating	No	No	No
Right-of-Way Cost Estimate	\$1,059,000	\$570,500	\$570,500
Total Estimated Cost	\$1,070,400	\$570,500	\$570,500

Table 9 SMF Alternative Matrix Analyses Basin 6

Notes:

• No floodplain compensation is required for Basin 6.

		SMF Site Alternatives	i i i i i i i i i i i i i i i i i i i
Alternative	7A	7B	7C
Location (Station) / Side (LT, RT)	1402+00/LT	1403+00/LT	1404+00/LT
SMF Area (acres)	1.5	1.7	1.5
Soils Names & Hydrologic Groups	Pomona (B/D)	Pomona (B/D)	Pomona (B/D)
Proximity to Outfall (feet)	0	0	0
Pipe Costs (Assume 36" Class II Conc. Pipe @ \$47/LF)	\$0	\$0	\$0
SMF Impacts to the 100-YR Floodplain (ac)	0	0	0
Right-of-Way Cost Estimate for 100- YR Floodplain Impact to SMF	\$0	\$0	\$0
Recorded Archaeological Sites	None	None	None
Impact to Section 4(f) Property	No	No	No
Wetlands	0	0	0
Wetland Mitigation Cost (\$95,000/acre)	\$0	\$0	\$0
Threatened and Endangered Species (Plant and Animals)	None	None	None
Contamination Risk Rating	No	No	No
Right-of-Way Cost Estimate	\$330,500	\$430,400	\$326,400
Total Estimated Cost	\$330,500	\$430,400	\$326,400

Table 10 SMF Alternative Matrix Analyses Basin 7

- No floodplain compensation is required for Basin 7.
- Variances in the SMF site alternative size are due to one or all of the following: differences in the estimated seasonal high water table elevation, estimated average ground elevations and/or parcel size.

	SMF Site Alternatives		
Alternative	8A	8B	8C
Location (Station) / Side (LT, RT)	1420+00/LT	1419+00/RT	1422+00/RT
SMF Area (acres)	2.2	2.2	2.2
Soils Names & Hydrologic Groups	Pomona (B/D) & Basinger (B/D)	Pomona (B/D)	Pomona (B/D) & Basinger (B/D)
Proximity to Outfall (feet)	0	0	0
Pipe Costs (Assume 36" Class II Conc. Pipe @ \$47/LF)	\$0	\$0	\$0
SMF Impacts to the 100-YR Floodplain (ac)	0	0	2.01
Right-of-Way Cost Estimate for 100-YR Floodplain Impact to SMF	\$0	\$0	\$396,700
Recorded Archaeological Sites	None	None	None
Impact to Section 4(f) Property	No	No	No
Wetlands	0	0	0
Wetland Mitigation Cost (\$95,000/acre)	\$0	\$0	\$0
Threatened and Endangered Species (Plant and Animals)	None	None	None
Contamination Risk Rating	No	No	No
Right-of-Way Cost Estimate	\$539,700	\$665,600	\$434,200
Total Estimated Cost	\$539,700	\$665,600	\$830,900

Table 11SMF Alternative Matrix AnalysesBasin 8

Notes:

• Basin 8 requires floodplain compensation. See Table 39 for floodplain compensation alternatives.

	SMF Site Alternatives		
Alternative	9A	9B	9C
Location (Station) / Side (LT, RT)	1426+00/LT	1428+00/RT	1428+00/LT
SMF Area (acres)	2.2	2.5	2.4
Soils Names & Hydrologic Groups	Basinger (B/D) & Lochloosa Fine Sand (C)	Pomona (B/D) & Basinger (B/D)	Lochloosa Fine Sand (C)
Proximity to Outfall (feet)	600	0	795
Pipe Costs (Assume 36" Class II Conc. Pipe @ \$47/LF)	\$28,200	\$0	\$37,370
SMF Impacts to the 100-YR Floodplain (ac)	0.26	2.42	0
Right-of-Way Cost Estimate for 100-YR Floodplain Impact to SMF	\$64,300	\$473,700	\$0
Recorded Archaeological Sites	None	None	None
Impact to Section 4(f) Property	No	No	No
Wetlands	0	0.41	0
Wetland Mitigation Cost (\$95,000/acre)	\$0	\$38,950	\$0
Threatened and Endangered Species (Plant and Animals)	None	None	None
Contamination Risk Rating	No	No	No
Right-of-Way Cost Estimate	\$442,800	\$473,700	\$595,500
Total Estimated Cost	\$535,300	\$986,350	\$632,870

Table 12 SMF Alternative Matrix Analyses Basin 9

Notes:

• Basin 9 requires floodplain compensation. See Table 39 for floodplain compensation alternatives.

	SI	MF Site Alternatives	
Alternative	10A	10B	10C
Location (Station) / Side (LT, RT)	1446+00/RT	1454+00/LT	1447+00/RT
SMF Area (acres)	4.1	4.1	4.1
Soils Names & Hydrologic Groups	Pomona (B/D)	Pomona (B/D)	Pomona (B/D)
Proximity to Outfall (feet)	0	50	0
Pipe Costs (Assume 36" Class II Conc. Pipe @ \$47/LF)	\$0	\$2350	\$0
SMF Impacts to the 100-YR Floodplain (ac)	4.09	0	4.09
Right-of-Way Cost Estimate for 100-YR Floodplain Impact to SMF	\$747,600	\$0	\$747,600
Recorded Archaeological Sites	None	None	None
Impact to Section 4(f) Property	No	No	No
Wetlands	0	0	0.52
Wetland Mitigation Cost (\$95,000/acre)	\$0	\$0	\$49,400
Threatened and Endangered Species (Plant and Animals)	None	None	Potential (Sandhill Crane Nest)
Contamination Risk Rating	No	No	No
Right-of-Way Cost Estimate	\$747,600	\$907,400	\$747,600
Total Estimated Cost	\$1,495,200	\$909,750	\$1,544,600

Table 13 SMF Alternative Matrix Analyses Basin 10

Notes:

• The impacts to the 100-year floodplain for Basin 10 will be compensated for in the preferred FPC site shown in Table 39.

	SMF Site Al	ternatives
Alternative	11A	11B
Location (Station) / Side (LT, RT)	1486+00/LT	1483+00/RT
SMF Area (acres)	1.8	1.7
Soils Names & Hydrologic Groups	Pomona (B/D), Sparr Fine Sand (C) & Blichton Fine Sand (D)	Sparr Fine Sand (C)
Proximity to Outfall (feet)	0	0
Pipe Costs (Assume 36" Class II Conc. Pipe @ \$47/LF)	\$0	\$0
SMF Impacts to the 100-YR Floodplain (ac)	0	0
Right-of-Way Cost Estimate for 100-YR Floodplain Impact to SMF	\$0	\$0
Recorded Archaeological Sites	None	None
Impact to Section 4(f) Property	No	No
Wetlands	0	0
Wetland Mitigation Cost (\$95,000/acre)	\$0	\$0
Threatened and Endangered Species (Plant and Animals)	None	None
Contamination Risk Rating	No	No
Right-of-Way Cost Estimate	\$384,800	\$347,300
Total Estimated Cost	\$384,800	\$347,300

Table 14SMF Alternative Matrix AnalysesBasin 11

- No floodplain compensation is required for Basin 11.
- Variances in the SMF site alternative size are due to one or all of the following: differences in the estimated seasonal high water table elevation, estimated average ground elevations and/or parcel size.

	SMF Site Alternatives		
Alternative	12A	12B	
Location (Station) / Side (LT, RT)	1499+00/LT	1499+00/RT	
SMF Area (acres)	1.7	1.7	
Soils Names & Hydrologic Groups	Pomona (B/D)	Pomona (B/D)	
Proximity to Outfall (feet)	0	0	
Pipe Costs (Assume 36" Class II Conc. Pipe @ \$47/LF)	\$0	\$0	
SMF Impacts to the 100-YR Floodplain (ac)	0	0	
Right-of-Way Cost Estimate for 100-YR Floodplain Impact to SMF	\$0	\$0	
Recorded Archaeological Sites	None	None	
Impact to Section 4(f) Property	No	No	
Wetlands	0	0	
Wetland Mitigation Cost (\$95,000/acre)	\$0	\$0	
Threatened and Endangered Species (Plant and Animals)	None	None	
Contamination Risk Rating	No	No	
Right-of-Way Cost Estimate	\$315,600	\$356,000	
Total Estimated Cost	\$315,600	\$356,000	

Table 15SMF Alternative Matrix AnalysesBasin 12

Notes:

• No floodplain compensation is required for Basin 12.

	SMF Site Alternatives		
Alternative	13A	13B	13C
Location (Station) / Side (LT, RT)	1547+00/RT	1543+00/LT	1548+00/LT
SMF Area (acres)	5.1	5.1	5.1
Soils Names & Hydrologic Groups	Sparr Fine Sand (C)	Sparr Fine Sand (C)	Sparr Fine Sand (C) & Blichton (D)
Proximity to Outfall (feet)	0	0	0
Pipe Costs (Assume 36" Class II Conc. Pipe @ \$47/LF)	\$0	\$0	\$0
SMF Impacts to the 100-YR Floodplain (ac)	0	0	0
Right-of-Way Cost Estimate for 100-YR Floodplain Impact to SMF	\$O	\$O	\$0
Recorded Archaeological Sites	None	None	None
Impact to Section 4(f) Property	No	No	No
Wetlands	0	0.22	0.29
Wetland Mitigation Cost (\$95,000/acre)	\$0	\$20,900	\$27,550
Threatened and Endangered Species (Plant and Animals)	None	None	None
Contamination Risk Rating	No	No	No
Right-of-Way Cost Estimate	\$6,076,100	\$1,717,100	\$1,697,800
Total Estimated Cost	\$6,076,100	\$1,738,000	\$1,725,350

Table 16SMF Alternative Matrix AnalysesBasin 13

Notes:

• No floodplain compensation is required for Basin 13.

	SMF Site Alternatives		
Alternative	14A	14B	14C
Location (Station) / Side (LT, RT)	1579+00/LT	1579+00/LT	1580+00/RT
SMF Area (acres)	2.2	2.6	2.1
Soils Names & Hydrologic Groups	Pompano Fine Sand (B/D), Blichton Fine Sand (D) & Sparr Fine Sand (C)	Blichton Fine Sand (D) & Sparr Fine Sand (C)	Pompano Fine Sand (B/D) & Sparr Fine Sand (C)
Proximity to Outfall (feet)	50	360	0
Pipe Costs (Assume 36" Class II Conc. Pipe @ \$47/LF)	\$2,350	\$16,920	\$0
SMF Impacts to the 100-YR Floodplain (ac)	0	0	0
Right-of-Way Cost Estimate for 100-YR Floodplain Impact to SMF	\$0	\$O	\$0
Recorded Archaeological Sites	None	None	None
Impact to Section 4(f) Property	No	No	No
Wetlands	0	0	0.35
Wetland Mitigation Cost (\$95,000/acre)	\$0	\$O	\$33,250
Threatened and Endangered Species (Plant and Animals)	None	None	None
Contamination Risk Rating	No	No	No
Right-of-Way Cost Estimate	\$4,474,500	\$4,493,300	\$1,751,300
Total Estimated Cost	\$4,476,850	\$4,510,220	\$1,784,550

Table 17SMF Alternative Matrix AnalysesBasin 14

Notes:

• No floodplain compensation is required for Basin 14.

Table 18			
SMF Alternative Matrix Analyses			
Basin 15			

	SMF Site Alternatives		
Alternative	15A	15B	15C
Location (Station) / Side (LT, RT)	1595+00/RT	1601+00/RT	1608+00/RT
SMF Area (acres)	7.8	8.8	6.8
Soils Names & Hydrologic Groups	Sparr Fine Sand (C) & Arredondo (B/D)	Sparr Fine Sand (C) & Pompano Fine Sand (B/D)	Sparr Fine Sand (C) & Pompano Fine Sand (B/D),
Proximity to Outfall (feet)	85	0	85
Pipe Costs (Assume 36" Class II Conc. Pipe @ \$47/LF)	\$3,995	\$O	\$3,995
SMF Impacts to the 100-YR Floodplain (ac)	0	4.10	0.31
Right-of-Way Cost Estimate for 100-YR Floodplain Impact to SMF	\$0	\$761,620	\$481,100
Recorded Archaeological Sites	None	None	None
Impact to Section 4(f) Property	No	No	No
Wetlands	0.37	2.63	0.45
Wetland Mitigation Cost (\$95,000/acre)	\$35,150	\$249,850	\$42,750
Threatened and Endangered Species (Plant and Animals)	None	None	None
Contamination Risk Rating	No	No	No
Right-of-Way Cost Estimate	\$1,457,700	\$1,463,800	\$11,996,100
Total Estimated Cost	\$1,496,845	\$2,475,270	\$12,519,950

Notes:

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No floodplain compensation is required for Basin 15.

	SMF Site Alternatives		
Alternative	16A	16B	
Location (Station) / Side (LT, RT)	1612+00/LT & RT	1615+00/LT	
SMF Area (acres)	7.9	7.7	
Soils Names & Hydrologic Groups	Sparr Fine Sand (C), Arredondo (B/D), Blitchton Fine Sand (D)	Arredondo (B/D) & Blitchton Fine Sand (D)	
Proximity to Outfall (feet)	0	0	
Pipe Costs (Assume 36" Class II Conc. Pipe @ \$47/LF)	\$0	\$O	
SMF Impacts to the 100-YR Floodplain (ac)	0	0	
Right-of-Way Cost Estimate for 100-YR Floodplain Impact to SMF	\$0	\$O	
Recorded Archaeological Sites	None	None	
Impact to Section 4(f) Property	No	No	
Wetlands	0	0	
Wetland Mitigation Cost (\$95,000/acre)	\$0	\$O	
Threatened and Endangered Species (Plant and Animals)	None	None	
Contamination Risk Rating	No	No	
Right-of-Way Cost Estimate	\$1,044,300	\$1,315,800	
Total Estimated Cost	\$1,044,300	\$1,315,800	

Table 19SMF Alternative Matrix AnalysesBasin 16

Notes:

• No floodplain compensation is required for Basin 16.

	SMF Site Alternatives	
Alternative	17A	17B
Location (Station) / Side (LT, RT)	1688+00/LT	1691+00/LT
SMF Area (acres)	8.5	8.7
Soils Names & Hydrologic Groups	Flemington (D), Kendrick Fine Sand (A) & Wachula (B/D)	Flemington (D), Kendrick Fine Sand (A) & Wachula (B/D)
Proximity to Outfall (feet)	115	185
Pipe Costs (Assume 36" Class II Conc. Pipe @ \$47/LF)	\$5400	\$8700
SMF Impacts to the 100-YR Floodplain (ac)	0	0.13
Right-of-Way Cost Estimate for 100-YR Floodplain Impact to SMF	\$0	\$16,950
Recorded Archaeological Sites	None	None
Impact to Section 4(f) Property	No	No
Wetlands	0.46	0
Wetland Mitigation Cost (\$95,000/acre)	\$43,700	\$0
Threatened and Endangered Species (Plant and Animals)	None	None
Contamination Risk Rating	No	No
Right-of-Way Cost Estimate	\$3,133,800	\$1,070,600
Total Estimated Cost	\$3,182,900	\$1,096,250

Table 20 SMF Alternative Matrix Analyses Basin 17

Notes:

• No floodplain compensation is required for Basin 17.

	SMF Site Alternatives	
Alternative	18A	18B
Location (Station) / Side (LT, RT)	1698+00/RT	1707+00/RT
SMF Area (acres)	7.6	7.6
Soils Names & Hydrologic Groups	Basinger (A/D) & Sparr Fine Sand (A)	Basinger (A/D) & Blichton (D)
Proximity to Outfall (feet)	85	100
Pipe Costs (Assume 36" Class II Conc. Pipe @ \$47/LF)	\$4000	\$4700
SMF Impacts to the 100-YR Floodplain (ac)	5.03	7.51
Right-of-Way Cost Estimate for 100-YR Floodplain Impact to SMF	\$1,891,300	\$1,177,800
Recorded Archaeological Sites	None	None
Impact to Section 4(f) Property	No	No
Wetlands	4.19	5.89
Wetland Mitigation Cost (\$95,000/acre)	\$398,050	\$559,550
Threatened and Endangered Species (Plant and Animals)	None	None
Contamination Risk Rating	No	No
Right-of-Way Cost Estimate	\$2,842,600	\$1,177,800
Total Estimated Cost	\$5,135,950	\$2,919,850

Table 21 SMF Alternative Matrix Analyses Basin 18

Notes:

• No floodplain compensation is required for Basin 18.

	SMF Site Alternatives		
Alternative	19A	19B	19C
Location (Station) / Side (LT, RT)	1765+00/LT	1763+00/LT	1763+00/LT
SMF Area (acres)	8.1	7.8	175 (Natural Discharge)
Soils Names & Hydrologic Groups	Arredondo Fine Sand (A), Nobleton Fine Sand (A) & Sparr Fine Sand (A)	Nobleton Fine Sand (A) & Sparr Fine Sand (A)	Arrendondo Fine Sand (A), Blichton (D), Kenapaha (A/D) & Sparr Fine Sand (A)
Proximity to Outfall (feet)	535	0	0
Pipe Costs (Assume 36" Class II Conc. Pipe @ \$47/LF)	\$25,150	\$0	\$0
SMF Impacts to the 100-YR Floodplain (ac)	3.89	0	0
Right-of-Way Cost Estimate for 100- YR Floodplain Impact to SMF	\$704,230	\$0	\$0
Recorded Archaeological Sites	None	None	None
Impact to Section 4(f) Property	Yes	Yes	Yes
Wetlands	0	0	0
Wetland Mitigation Cost (\$95,000/acre)	\$O	\$0	\$0
Threatened and Endangered Species (Plant and Animals)	None	None	None
Contamination Risk Rating	No	No	No
Right-of-Way Cost Estimate	\$1,397,600	\$179,500	\$26,540,200
Total Estimated Cost	\$2,126,980	\$179,500	\$26,540,200

Table 22SMF Alternative Matrix AnalysesBasin 19

- No floodplain compensation is required for Basin 19.
- Variances in the SMF site alternative size are due to one or all of the following: differences in the estimated seasonal high water table elevation, estimated average ground elevations and/or parcel size.

	SMF Site Alternatives		
Alternative	20A	20B	20C
Location (Station) / Side (LT, RT)	1790+00/LT	1794+00/LT	1788+00/LT
SMF Area (acres)	4.2	4.7	5.2
Soils Names & Hydrologic Groups	Blichton Fine Sand (D)	Blichton Fine Sand (D)	Blichton Fine Sand (D) & Sparr Fine Sand (A)
Proximity to Outfall (feet)	0	510	145
Pipe Costs (Assume 36" Class II Conc. Pipe @ \$47/LF)	\$0	\$23,970	\$6,820
SMF Impacts to the 100-YR Floodplain (ac)	0	0	0
Right-of-Way Cost Estimate for 100-YR Floodplain Impact to SMF	\$O	\$0	\$0
Recorded Archaeological Sites	None	None	None
Impact to Section 4(f) Property	No	No	No
Wetlands	1.76	0	0
Wetland Mitigation Cost (\$95,000/acre)	\$167,200	\$0	\$0
Threatened and Endangered Species (Plant and Animals)	None	None	None
Contamination Risk Rating	No	No	No
Right-of-Way Cost Estimate	\$779,400	\$861,500	\$941,000
Total Estimated Cost	\$946,600	\$885,470	\$947,820

Table 23SMF Alternative Matrix AnalysesBasin 20

Notes:

• No floodplain compensation is required for Basin 20.

	SMF Site Alternatives		
Alternative	21A	21B	21C
Location (Station) / Side (LT, RT)	1823+00/LT	1825+00/RT	1823+00/LT
SMF Area (acres)	6.8	6.7	6.8
Soils Names & Hydrologic Groups	Candler Fine Sand (A) & Sparr Fine Sand (A)	Candler Fine Sand (A) & Sparr Fine Sand (A)	Arredondo Fine Sand (A), Candler Fine Sand (A) & Sparr Fine Sand (A)
Proximity to Outfall (feet)	0	0	70
Pipe Costs (Assume 36" Class II Conc. Pipe @ \$47/LF)	\$0	\$0	\$3,290
SMF Impacts to the 100-YR Floodplain (ac)	0	0	0
Right-of-Way Cost Estimate for 100-YR Floodplain Impact to SMF	\$0	\$0	\$0
Recorded Archaeological Sites	None	None	None
Impact to Section 4(f) Property	No	No	No
Wetlands	0	0	0
Wetland Mitigation Cost (\$95,000/acre)	\$0	\$0	\$0
Threatened and Endangered Species (Plant and Animals)	None	None	None
Contamination Risk Rating	No	No	No
Right-of-Way Cost Estimate	\$1,172,200	\$995,000	\$1,183,900
Total Estimated Cost	\$1,172,200	\$995,000	\$1,187,190

Table 24SMF Alternative Matrix AnalysesBasin 21

Notes:

• No floodplain compensation is required for Basin 21.

	SMF Site Alternatives		
Alternative	22A	22B	22C
Location (Station) / Side (LT, RT)	1862+00/LT	1858+00/LT	1864+00/LT
SMF Area (acres)	7.1	7.2	7.3
Soils Names & Hydrologic Groups	Arredondo Fine Sand (A) & Candler Fine Sand (A)	Arredondo Fine Sand (A), Blichton Fine Sand (D) & Candler Fine Sand (A)	Arredondo Fine Sand (A) & Candler Fine Sand (A)
Proximity to Outfall (feet)	0	0	0
Pipe Costs (Assume 36" Class II Conc. Pipe @ \$47/LF)	\$0	\$0	\$0
SMF Impacts to the 100-YR Floodplain (ac)	0	0	0
Right-of-Way Cost Estimate for 100-YR Floodplain Impact to SMF	\$0	\$0	\$0
Recorded Archaeological Sites	None	None	None
Impact to Section 4(f) Property	No	No	No
Wetlands	0	0	0
Wetland Mitigation Cost (\$95,000/acre)	\$0	\$0	\$0
Threatened and Endangered Species (Plant and Animals)	None	None	None
Contamination Risk Rating	No	No	No
Right-of-Way Cost Estimate	\$1,164,800	\$2,312,100	\$1,251,300
Total Estimated Cost	\$1,164,800	\$2,312,100	\$1,251,300

Table 25SMF Alternative Matrix AnalysesBasin 22

Notes:

• No floodplain compensation is required for Basin 22.

	SMF Site Alternatives		
Alternative	23A	23B	23C
Location (Station) / Side (LT, RT)	1896+00/RT	1900+00/LT	1895+00/LT
SMF Area (acres)	4.8	5.1	4.9
Soils Names & Hydrologic Groups	Candler Fine Sand (A)	Candler Fine Sand (A)	Candler Fine Sand (A)
Proximity to Outfall (feet)	0	0	0
Pipe Costs (Assume 36" Class II Conc. Pipe @ \$47/LF)	\$0	\$0	\$0
SMF Impacts to the 100-YR Floodplain (ac)	0	0	0
Right-of-Way Cost Estimate for 100-YR Floodplain Impact to SMF	\$0	\$0	\$0
Recorded Archaeological Sites	None	None	None
Impact to Section 4(f) Property	No	No	No
Wetlands	0	0	0
Wetland Mitigation Cost (\$95,000/acre)	\$0	\$O	\$0
Threatened and Endangered Species (Plant and Animals)	None	None	None
Contamination Risk Rating	No	No	No
Right-of-Way Cost Estimate	\$741,600	\$768,700	\$752,100
Total Estimated Cost	\$741,600	\$768,700	\$752,100

Table 26SMF Alternative Matrix AnalysesBasin 23

Notes:

• No floodplain compensation is required for Basin 23.

	SMF Site Alternatives		
Alternative	24A	24B	24C
Location (Station) / Side (LT, RT)	1941+00/LT	1933+00/LT	1935+00/RT
SMF Area (acres)	7.4	7.1	7.3
Soils Names & Hydrologic Groups	Arredondo Fine Sand (A) & Candler Fine Sand (A)	Candler Fine Sand (A)	Candler Fine Sand (A)
Proximity to Outfall (feet)	0	0	0
Pipe Costs (Assume 36" Class II Conc. Pipe @ \$47/LF)	\$0	\$0	\$0
SMF Impacts to the 100-YR Floodplain (ac)	0	0	0
Right-of-Way Cost Estimate for 100-YR Floodplain Impact to SMF	\$O	\$0	\$0
Recorded Archaeological Sites	None	None	None
Impact to Section 4(f) Property	No	No	No
Wetlands	0	0	0
Wetland Mitigation Cost (\$95,000/acre)	\$O	\$0	\$0
Threatened and Endangered Species (Plant and Animals)	None	None	None
Contamination Risk Rating	No	No	No
Right-of-Way Cost Estimate	\$1,077,400	\$1,051,400	\$1,053,200
Total Estimated Cost	\$1,077,400	\$1,051,400	\$1,053,200

Table 27SMF Alternative Matrix AnalysesBasin 24

Notes:

• No floodplain compensation is required for Basin 24.

	SMF Site Alternatives		
Alternative	25A	25B	25C
Location (Station) / Side (LT, RT)	1982+00/LT	1987+00/RT	1987+00/LT
SMF Area (acres)	5.0	4.5	5.1
Soils Names & Hydrologic Groups	Arredondo Fine Sand (A)	Arredondo Fine Sand (A) & Candler Fine Sand (A)	Arredondo Fine Sand (A) & Candler Fine Sand (A)
Proximity to Outfall (feet)	0	0	0
Pipe Costs (Assume 36" Class II Conc. Pipe @ \$47/LF)	\$0	\$0	\$0
SMF Impacts to the 100-YR Floodplain (ac)	0	0	0
Right-of-Way Cost Estimate for 100-YR Floodplain Impact to SMF	\$O	\$O	\$0
Recorded Archaeological Sites	None	None	None
Impact to Section 4(f) Property	No	No	No
Wetlands	0	0	0
Wetland Mitigation Cost (\$95,000/acre)	\$O	\$O	\$0
Threatened and Endangered Species (Plant and Animals)	None	None	None
Contamination Risk Rating	No	No	No
Right-of-Way Cost Estimate	\$1,259,300	\$1,685,300	\$766,200
Total Estimated Cost	\$1,259,300	\$1,685,300	\$766,200

Table 28SMF Alternative Matrix AnalysesBasin 25

Notes:

• No floodplain compensation is required for Basin 25.

	SMF Site Alternatives		
Alternative	26A	26B	26C
Location (Station) / Side (LT, RT)	2003+00/LT	2002+00/LT	2006+00/RT
SMF Area (acres)	6.0	6.0	5.2
Soils Names & Hydrologic Groups	Candler Fine Sand (A)	Candler Fine Sand (A)	Candler Fine Sand (A)
Proximity to Outfall (feet)	0	0	0
Pipe Costs (Assume 36" Class II Conc. Pipe @ \$47/LF)	\$0	\$0	\$0
SMF Impacts to the 100-YR Floodplain (ac)	0	0	0
Right-of-Way Cost Estimate for 100-YR Floodplain Impact to SMF	\$O	\$O	\$0
Recorded Archaeological Sites	None	None	None
Impact to Section 4(f) Property	No	No	No
Wetlands	0	0	0
Wetland Mitigation Cost (\$95,000/acre)	\$O	\$O	\$0
Threatened and Endangered Species (Plant and Animals)	None	None	None
Contamination Risk Rating	No	No	No
Right-of-Way Cost Estimate	\$886,000	\$884,000	\$789,400
Total Estimated Cost	\$886,000	\$884,000	\$789,400

Table 29SMF Alternative Matrix AnalysesBasin 26

Notes:

• No floodplain compensation is required for Basin 26.

	SMF Site Alternatives		
Alternative	27A	27B	27C
Location (Station) / Side (LT, RT)	2037+00/RT	2028+00/LT	2028+00/RT
SMF Area (acres)	3.9	3.8	3.7
Soils Names & Hydrologic Groups	Candler Fine Sand (A)	Candler Fine Sand (A)	Candler Fine Sand (A)
Proximity to Outfall (feet)	0	0	0
Pipe Costs (Assume 36" Class II Conc. Pipe @ \$47/LF)	\$0	\$0	\$0
SMF Impacts to the 100-YR Floodplain (ac)	0	0	0
Right-of-Way Cost Estimate for 100-YR Floodplain Impact to SMF	\$O	\$O	\$0
Recorded Archaeological Sites	None	None	None
Impact to Section 4(f) Property	No	No	No
Wetlands	0	0	0
Wetland Mitigation Cost (\$95,000/acre)	\$O	\$O	\$0
Threatened and Endangered Species (Plant and Animals)	None	None	None
Contamination Risk Rating	No	No	No
Right-of-Way Cost Estimate	\$1,346,400	\$3,143,800	\$588,900
Total Estimated Cost	\$1,346,400	\$3,143,800	\$588,900

Table 30SMF Alternative Matrix AnalysesBasin 27

Notes:

• No floodplain compensation is required for Basin 27.

	SMF Site Alternatives		
Alternative	29A	29B	29C
Location (Station) / Side (LT, RT)	2066+00/RT	2065+00/LT	2068+00/RT
SMF Area (acres)	7.3	7.2	7.6
Soils Names & Hydrologic Groups	Candler Fine Sand (A) & Tavares (A)	Candler Fine Sand (A)	Candler Fine Sand (A) & Tavares (A)
Proximity to Outfall (feet)	370	100	0
Pipe Costs (Assume 36" Class II Conc. Pipe @ \$47/LF)	\$17,390	\$4,700	\$0
SMF Impacts to the 100-YR Floodplain (ac)	0	0	0
Right-of-Way Cost Estimate for 100-YR Floodplain Impact to SMF	\$O	\$O	\$0
Recorded Archaeological Sites	None	None	None
Impact to Section 4(f) Property	No	Yes	No
Wetlands	0	0	0
Wetland Mitigation Cost (\$95,000/acre)	\$O	\$O	\$0
Threatened and Endangered Species (Plant and Animals)	None	None	None
Contamination Risk Rating	No	No	No
Right-of-Way Cost Estimate	\$4,183,000	\$536,500	\$3,287,700
Total Estimated Cost	\$4,200,390	\$541,200	\$3,287,700

Table 31SMF Alternative Matrix AnalysesBasin 29

- ¹The cost of the least expensive alternative, 29B, is located on forestry lands and therefore not selected as the preferred. SMF 29C is the preferred since it is an avoidance alternative to a 4(f) property. The right-of-way costs was not considered a critical factor in the selection of the preferred SMF alternative.
- No floodplain compensation is required for Basin 29.
- Variances in the SMF site alternative size are due to one or all of the following: differences in the estimated seasonal high water table elevation, estimated average ground elevations and/or parcel size.

Table 32 SMF Alternative Matrix Analyses Basin 30

	SMF Site Alternatives			
Alternative	30A	30B	30C & 3a/3bC	30D & 3a/3bC
Location (Station) / Side (LT, RT)	2170+00/RT	2162+00/RT	2169+00/RT & 2116+00/RT	2163+00/RT & 2116+00/RT
SMF Area (acres)	14.4	15.0	25.1 (SMF/Natural Discharge)	25.7 (SMF/Natural Discharge)
Soils Names & Hydrologic Groups	Lake Fine Sand (A) & Pits (N/A)	Lake Fine Sand (A) & Sparr Fine Sand (A)	Candler Fine Sane (A) & Lake Fine Sand (A)	Lake Fine Sand (A) & Sparr Fine Sand (A)
Proximity to Outfall (feet)	0	540	0	700
Pipe Costs (Assume 36" Class II Conc. Pipe @ \$47/LF)	\$0	\$25,380	\$0	\$32,900
SMF Impacts to the 100-YR Floodplain (ac)	0	0	0	0
Right-of-Way Cost Estimate for 100-YR Floodplain Impact to SMF	\$O	\$0	\$O	\$O
Recorded Archaeological Sites	None	None	None	None
Impact to Section 4(f) Property	No	No	Yes	Yes
Wetlands	0	0	0	0
Wetland Mitigation Cost (\$95,000/acre)	\$0	\$0	\$0	\$O
Threatened and Endangered Species (Plant and Animals)	None	None	None	None
Contamination Risk Rating	No	No	No	No
Right-of-Way Cost Estimate	\$8,188,300	\$7,643,900	\$7,799,100	\$7,613,100
Total Estimated Cost	\$8,188,300	\$7,669,280	\$7,799,100	\$7,646,000

Notes:

• No floodplain compensation is required for Basin 30.

Table 33SMF Alternative Matrix AnalysesBasin 31

	SMF Site Alternatives			
Alternative	31A	31B	4a(e)C, 4a(w)C, 4b(e)C &4b(w)C	31D, 4b(e)C &4b(w)C
Location (Station) / Side (LT, RT)	2228+00/RT	2227+00/RT	2205+00/RT, 2212+00/LT, 2233+00/RT & 2233+00/LT	2200+00/RT, 2233+00/RT & 2233+00/LT
SMF Area (acres)	8.3	8.0	17.6 (Natural Discharge)	13.9 (SMF/Natural Discharge)
Soils Names & Hydrologic Groups	Pits (N/A)	Candler Fine Sand (A) & Sparr Fine Sand (A)	Arredondo Fine Sand (A), Candler Fine Sand (A), Lake Fine Sand (A) & Sparr Fine Sand (A)	Candler Fine Sand (A), Lake Fine Sand (A) & Sparr Fine Sand (A)
Proximity to Outfall (feet)	465	0	0	850
Pipe Costs (Assume 36" Class II Conc. Pipe @ \$47/LF)	\$21,860	\$0	\$0	\$39,960
SMF Impacts to the 100-YR Floodplain (ac)	0	0	0	0
Right-of-Way Cost Estimate for 100-YR Floodplain Impact to SMF	\$0	\$0	\$0	\$0
Recorded Archaeological Sites	None	None	None	None
Impact to Section 4(f) Property	Yes	Yes	Yes	Yes
Wetlands	0	0	0	0
Wetland Mitigation Cost (\$95,000/acre)	\$0	\$0	\$0	\$0
Threatened and Endangered Species (Plant and Animals)	None	None	None	None
Contamination Risk Rating	No	No	No	No
Right-of-Way Cost Estimate	\$378,600	\$365,800	\$856,800	\$1,209,500
Total Estimated Cost	\$400,460	\$365,800	\$856,800	\$1,249,460 ¹

- ¹Actual cost of acquisitions and/or easements within forestry lands may be subject to a negotiated mitigation package. Since the right-of-way costs may not be indicative of the final mitigation costs; this criterion was not considered a critical factor in the selection of the preferred SMF alternative. (See Section 9.0 for further discussion.)
- No floodplain compensation is required for Basin 31.
- Variances in the SMF site alternative size are due to one or all of the following: differences in the estimated seasonal high water table elevation, estimated average ground elevations and/or parcel size.

	SMF Site Alternatives			
Alternative	32A	32B	5aC & 5bC	
Location (Station) / Side (LT, RT)	2250+00/RT	2260+00/LT	2252+00/LT & 2265+00/LT	
SMF Area (acres)	7.0	6.4	12.0 (Natural Discharge)	
Soils Names & Hydrologic Groups	Pits (N/A)	Candler Fine Sand (A) & Sparr Fine Sand (A)	Basinger (A/D), Candler Fine Sand (A) & Sparr Fine Sand (A)	
Proximity to Outfall (feet)	270	0	0	
Pipe Costs (Assume 36" Class II Conc. Pipe @ \$47/LF)	\$12,690	\$0	\$0	
SMF Impacts to the 100-YR Floodplain (ac)	0	0	0	
Right-of-Way Cost Estimate for 100-YR Floodplain Impact to SMF	\$0	\$0	\$0	
Recorded Archaeological Sites	None	None	None	
Impact to Section 4(f) Property	Yes	Yes	Yes	
Wetlands	0	0	0	
Wetland Mitigation Cost (\$95,000/acre)	\$0	\$0	\$0	
Threatened and Endangered Species (Plant and Animals)	None	None	None	
Contamination Risk Rating	No	No	No	
Right-of-Way Cost Estimate	\$321,100	\$298,900	\$289,700	
Total Estimated Cost	\$333,790	\$298,900	\$289,700 ¹	

Table 34SMF Alternative Matrix AnalysesBasin 32

- ¹Actual cost of acquisitions and/or easements within forestry lands will be subject to a negotiated mitigation package. Since the right-of-way costs may not be indicative of the final mitigation costs; this criterion was not considered a critical factor in the selection of the preferred SMF alternative.
- No floodplain compensation is required for Basin 32.
- Variances in the SMF site alternative size are due to one or all of the following: differences in the estimated seasonal high water table elevation, estimated average ground elevations and/or parcel size.

	SMF Site Alternatives		
Alternative	33A	33B	6a/bC & 6cC
Location (Station) / Side (LT, RT)	2303+00/LT	2300+00/LT	2300+00/RT & 2339+00/RT
SMF Area (acres)	13.9	13.6	9.4 (Natural Discharge)
Soils Names & Hydrologic Groups	Lake (A) & Pits (N/A)	Lake (A) & Sumterville (C)	Lake (A) & Sumterville (C)
Proximity to Outfall (feet)	410	0	0
Pipe Costs (Assume 36" Class II Conc. Pipe @ \$47/LF)	\$19,270	\$0	\$0
SMF Impacts to the 100-YR Floodplain (ac)	0	0	0
Right-of-Way Cost Estimate for 100-YR Floodplain Impact to SMF	\$0	\$0	\$0
Recorded Archaeological Sites	None	None	None
Impact to Section 4(f) Property	Yes	Yes	Yes
Wetlands	0	0	0
Wetland Mitigation Cost (\$95,000/acre)	\$0	\$0	\$0
Threatened and Endangered Species (Plant and Animals)	None	None	None
Contamination Risk Rating	No	No	No
Right-of-Way Cost Estimate	\$615,700	\$604,000	\$237,000
Total Estimated Cost	\$634,970	\$604,000	\$237,000 ¹

Table 35SMF Alternative Matrix AnalysesBasin 33

- ¹Actual cost of acquisitions and/or easements within forestry lands will be subject to a negotiated mitigation package. Since the right-of-way costs may not be indicative of the final mitigation costs; this criterion was not considered a critical factor in the selection of the preferred SMF alternative.
- No floodplain compensation is required for Basin 33.
- Variances in the SMF site alternative size are due to one or all of the following: differences in the estimated seasonal high water table elevation, estimated average ground elevations and/or parcel size.

	SMF Site Alternatives		
Alternative	34A	34B	7C
Location (Station) / Side (LT, RT)	2353+00/RT	2342+00/LT	2345+00/LT
SMF Area (acres)	8.3	10.2	7.4 (Natural Discharge)
Soils Names & Hydrologic Groups	Candler Fine Sand (A), Adamsville (C) & Sumterville Fine Sand (C)	Pits (N/A)	Candler Fine Sand (A) & Sumterville Fine Sand (C)
Proximity to Outfall (feet)	0	600	0
Pipe Costs (Assume 36" Class II Conc. Pipe @ \$47/LF)	\$0	\$28,200	\$0
SMF Impacts to the 100-YR Floodplain (ac)	0	0	0
Right-of-Way Cost Estimate for 100-YR Floodplain Impact to SMF	\$0	\$0	\$0
Recorded Archaeological Sites	None	None	None
Impact to Section 4(f) Property	Yes	Yes	Yes
Wetlands	0	0	0
Wetland Mitigation Cost (\$95,000/acre)	\$0	\$0	\$0
Threatened and Endangered Species (Plant and Animals)	None	None	None
Contamination Risk Rating	No	No	No
Right-of-Way Cost Estimate	\$377,900	\$481,000	\$172,500
Total Estimated Cost	\$377,900	\$509,200	\$172,500 ¹

Table 36SMF Alternative Matrix AnalysesBasin 34

- ¹Actual cost of acquisitions and/or easements within forestry lands will be subject to a negotiated mitigation package. Since the right-of-way costs may not be indicative of the final mitigation costs; this criterion was not considered a critical factor in the selection of the preferred SMF alternative.
- No floodplain compensation is required for Basin 34.
- Variances in the SMF site alternative size are due to one or all of the following: differences in the estimated seasonal high water table elevation, estimated average ground elevations and/or parcel size.
| | FPC Site Alt | ernatives |
|---|----------------------------|-------------------------|
| Alternative | Portion of SMF
3A | Adjacent to
SMF 4A |
| Location (Station) / Side (LT, RT) | 1284+00/LT | 1303+00/RT |
| Soils Names & Hydrologic Groups | Lochloosa Fine
Sand (C) | Newnan Fine
Sand (C) |
| Estimated Depth to SHWT (ft) | 3.75 | 2.0 |
| Preferred SMF Impacts to the 100-
YR Floodplain (ac-ft) | 1.97 | 1.97 |
| Right-of-way Impacts to the 100-YR
Floodplain (ac-ft) | 0.66 | 0.66 |
| Total Impacts to the 100-YR
Floodplain (ac-ft) | 2.63 | 2.63 |
| FPC Area (acres) | 0.70 | 1.32 |
| Total Right-of-Way Cost Estimate
for 100-YR Floodplain Impacts | \$144,350 | \$518,450 |
| Recorded Archaeological Sites | None | None |
| Wetlands | 0 | 0 |
| Wetland Mitigation Cost
(\$95,000/acre) | \$0 | \$0 |
| Threatened and Endangered Species (Plant and Animals) | None | None |
| Contamination Risk Rating | No | No |
| Total Estimated Cost | \$144,350 | \$518,450 |

Table 37FPC Alternative Matrix AnalysesBasins 3 & 4

Alternative	Adjacent to SMF 5C
Location (Station) / Side (LT, RT)	1339+00/RT
Soils Names & Hydrologic Groups	Newnan Fine Sand (C)
Estimated Depth to SHWT (ft)	2.0
Preferred SMF Impacts to the 100- YR Floodplain (ac-ft)	0.20
Right-of-way Impacts to the 100-YR Floodplain (ac-ft)	0.18
Total Impacts to the 100-YR Floodplain (ac-ft)	0.38
FPC Area (acres)	0.2
Total Right-of-Way Cost Estimate for 100-YR Floodplain Impacts	\$30,050
Recorded Archaeological Sites	None
Wetlands	0
Wetland Mitigation Cost (\$95,000/acre)	\$0
Threatened and Endangered Species (Plant and Animals)	None
Contamination Risk Rating	No
Total Estimated Cost	\$30,050

Table 38FPC Alternative Matrix AnalysesBasin 5

	FPC Site Alt	ernatives
Alternative	Portion of SMF 8B	Portion of SMF 9C
Location (Station) / Side (LT, RT)	1419+00/RT	1428+00/LT
Soils Names & Hydrologic Groups	Pomona (B/D)	Lochloosa Fine Sand (C)
Estimated Depth to SHWT (ft)	0.5	2.5
Preferred SMF Impacts to the 100- YR Floodplain (ac-ft)	0.26	0.26
Right-of-way Impacts to the 100-YR Floodplain (ac-ft)	1.51	1.51
Total Impacts to the 100-YR Floodplain (ac-ft)	1.77	1.77
FPC Area (acres)	3.5	0.7
Total Right-of-Way Cost Estimate for 100-YR Floodplain Impacts	\$1,066,100	\$236,200
Recorded Archaeological Sites	None	None
Wetlands	0	0
Wetland Mitigation Cost (\$95,000/acre)	\$0	\$0
Threatened and Endangered Species (Plant and Animals)	None	None
Contamination Risk Rating	No	No
Total Estimated Cost	\$1,066,100	\$236,200

Table 39FPC Alternative Matrix AnalysesBasins 8, 9 & 10

Alternative	Portion of SMF 18A
Location (Station) / Side (LT, RT)	1698+00/RT
Soils Names & Hydrologic Groups	Sparr Fine Sand (A)
Estimated Depth to SHWT (ft)	2.5
Preferred SMF Impacts to the 100- YR Floodplain (ac-ft)	7.51
Right-of-way Impacts to the 100-YR Floodplain (ac-ft)	0.0
Total Impacts to the 100-YR Floodplain (ac-ft)	7.51
FPC Area (acres)	3.0
Total Right-of-Way Cost Estimate for 100-YR Floodplain Impacts	\$470,500
Recorded Archaeological Sites	None
Wetlands	0
Wetland Mitigation Cost (\$95,000/acre)	\$0
Threatened and Endangered Species (Plant and Animals)	None
Contamination Risk Rating	No
Total Estimated Cost	\$470,500

Table 40FPC Alternative Matrix AnalysesBasin 18

9.0 **RECOMMENDATIONS**

Table 41 summarizes the preferred SMF and FPC sites for the proposed project.

Preferred SMF / FPC Sites	Station - Location	Area (ac)
SMF 2A	1255+00, RT	2.7
SMF 3C	1281+00, RT	2.2
FPC 3A	1284+00, LT	0.7
SMF 4B	1298+00, RT	4.1
SMF 5C	1339+00, RT	4.3
FPC (Adjacent to SMF) 5C	1339+00, RT	0.2
SMF 6C	1380+00, LT	3.1
SMF 7C	1404+00, LT	1.5
SMF 8A	1420+00, LT	2.2
SMF 9A	1426+00, LT	2.2
FPC 9C	1428+00, LT	0.7
SMF 10B	1454+00, LT	4.1
SMF 11B	1483+00, RT	1.7
SMF 12A	1499+00, LT	1.7
SMF 13C	1548+00, LT	5.1
SMF 14C	1580+00, RT	2.1
SMF 15A	1595+00, RT	7.8
SMF 16A	1612+00, LT & RT	7.9
SMF 17B	1691+00, LT	8.7
SMF 18B	1707+00, RT	7.5
FPC 18A	1698+00, RT	3.0
SMF 19B	1763+00, LT	7.8
SMF 20B	1794+00, LT	4.7
SMF 21B	1825+00, RT	6.7
SMF 22A	1862+00, LT	7.1
SMF 23A	1896+00, RT	4.8

Table 41Preferred SMF and FPC Sites

Preferred SMF / FPC Sites	Station - Location	Area (ac)
SMF 24B	1933+00, LT	7.1
SMF 25C	1987+00, LT	5.1
SMF 26C	2006+00, RT	5.2
SMF 27C	2028+00, RT	3.7
SMF 29C	2068+00, RT	7.6
SMF 30B	2162+00, RT	15.0
SMF 31D, 4b(e)C & 4b(w)C	2200+00, RT; 2233+00, RT & 2233+00, LT	13.9 (20.4 Easement) ⁽¹⁾
5aC & 5bC	2252+00, LT & 2265+00, LT	12.0 (53.1 Easement) ⁽¹⁾
6a/bC & 6cC	2300+00, RT & 2339+00, RT	9.4 (37.3 Easement) ⁽¹⁾
7C	2345+00, LT	7.4 (10.8 Easement) ⁽¹⁾

(1) Department intends to acquire Perpetual Transportation/Drainage/Maintenance Easements within the Withlacoochee State Forest. These easements will include areas of stormwater conveyance.

BASIN 1 Not used.

BASIN 2

The preferred SMF Site 2A is 2.7 ac and is located just north of Oscie Murphey Road at Sta. 1255+00 (RT). The total estimated cost for this site is \$504,355. Basin 2 does not require floodplain compensation. Based on land use, proximity to outfall, and lowest cost, SMF Site 2A was selected as the preferred alternative.

BASIN 3

The preferred SMF site is 3C and is approximately 2.2 ac. It is located at Sta. 1281+00 (RT). The total estimated cost is \$392,700. There are impacts to the 100-year floodplain within the right-of-way in basins 3 and 4. In order to meet the required floodplain compensation in this area, it is necessary to also acquire a floodplain compensation site. Floodplain compensation for Basins 3 and 4 can be accommodated in a portion of the SMF site 3A, approximately 0.7 acres, and is located at Sta. 1284+00 (LT). The estimated cost for FPC 3A is \$144,350. Based on land use, proximity to outfall, and lowest cost, the preferred alternatives for Basin 3 is SMF Site 3C and FPC Site 3A.

BASIN 4

The preferred SMF site is 4B and is approximately 4.1 ac with an estimated cost of \$958,130. This SMF is located at Sta. 1298+00 (RT). The 100-year floodplain impacts associated with Basin 4 within the right-of-way as well as the impacts due to the preferred SMF Site 4B will be compensated in FPC Site 3A which is described in the recommendation

section for Basin 3. Based on land use, proximity to outfall, and lowest cost, the preferred alternative for Basin 4 is SMF Site 4B.

BASIN 5

The preferred SMF site is approximately 4.3 ac with an estimated cost of \$692,330. This SMF site is located at Sta. 1339+00 (RT) north of Darby Road. Basin 5 does encroach into the 100-year floodplain within the right-of-way and therefore, does require floodplain compensation. Floodplain compensation will be accomplished adjacent to the preferred SMF 5C. Since this impact is minimal, approximately 0.2 ac additional acres will be required. The estimated cost for this compensation is \$30,050. Based on proximity to outfall, and the lowest cost, the preferred alternative for Basin 5 is SMF Site 5C.

BASIN 6

The preferred SMF site is 6C and is approximately 3.1 ac with an estimated cost of \$570,500. It is located at Sta. 1380+00 (LT) adjacent to Cross Drain No. 4. Alternative 6B is also on the same parcel as SMF Site 6C and has the same estimated cost. The preferred site, 6C, is orientated along the Cross Drain No. 4's outfall and is more hydraulically preferable than 6B. The existing contours are lower on the western side of 6C and the existing average ground elevation of 6B is slightly higher than 6C. Basin 6 does not require floodplain compensation. Based on the reasons listed above, the preferred alternative for Basin 6 is SMF Site 6C.

<u>BASIN 7</u>

The preferred SMF site is 7C and is approximately 1.5 ac with an estimated cost of \$326,400. This SMF is located south of CR 578 at Sta. 1404+00 (LT). Basin 7 does not require floodplain compensation. Based on proximity to outfall, and lowest cost, the preferred alternative for Basin 7 is SMF Site 7C.

BASIN 8

The preferred SMF site is 8A and is approximately 2.2 ac. It is located at Sta. 1420+00 (LT). The total estimated cost is \$539,700. There are impacts to the 100-year floodplain within the right-of-way in basins 8, 9 and 10 which will be compensated for in Basin 9. Based on land use, proximity to outfall, and lowest cost, the preferred alternatives for Basin 8 is SMF Site 8A.

<u>BASIN 9</u>

The preferred SMF site is 9A and is approximately 2.2 ac. It is located at Sta. 1426+00 (LT). The total estimated cost is \$535,300. There are impacts to the 100-year floodplain within the right-of-way in basins 8 and 9 and impacts to the preferred SMF 9A. In order to meet the required floodplain compensation in this area, it is necessary to also acquire a floodplain compensation site. Floodplain compensation for Basins 8, 9 and 10 can be accommodated in a portion of the SMF site 9C, approximately 0.7 acres, and is located at Sta. 1428+00 (LT) which is adjacent to SMF 9A. The estimated cost for FPC 9C is \$236,200. Based on land use, proximity to outfall, and lowest cost, the preferred alternatives for Basin 9 is SMF Site 9A and FPC Site 9C.

<u>BASIN 10</u>

The preferred SMF site is 10B and is approximately 4.1 ac. It is located at Sta. 1454+00 (LT) near Cross Drain No. 8. The total estimated cost is \$909,750. There are impacts to the 100-year floodplain within the right-of-way in basin 10 which will be compensated for in Basin 9. Based on land use, proximity to outfall, and lowest cost, the preferred alternatives for Basin 10 is SMF Site 10B.

<u>BASIN 11</u>

The preferred SMF site is 11B and is approximately 1.7 ac with an estimated cost of \$347,300. This SMF is located at Sta. 1483+00 (RT) adjacent to Cross Drain No. 9. Basin 11 does not require floodplain compensation. Based on proximity to outfall, and lowest cost, the preferred alternative for Basin 11 is SMF Site 11B.

BASIN 12

The preferred SMF site is 12A and is approximately 1.7 ac with an estimated cost of \$315,600. This SMF is located at Sta. 1499+00 (LT) adjacent to Cross Drain No. 10 and near Lake Iola Rd. Basin 12 does not require floodplain compensation. Based on proximity to outfall, and lowest cost, the preferred alternative for Basin 12 is SMF Site 12A.

BASIN 13

The preferred SMF site is 13C and is approximately 5.1 ac with an estimated cost of \$1,725,350. This SMF is located at Sta. 1548+00 (LT) adjacent to Cross Drain No. 11 and south of Moody Lake. Although the 100-year Floodplain abuts I-75 in this basin, measures will be implemented in the design phase to minimize any impacts to the 100-year floodplain along Moody Lake; therefore, Basin 13 does not require floodplain compensation. Based on proximity to outfall, and lowest cost, the preferred alternative for Basin 13 is SMF Site 13C.

BASIN 14

The preferred SMF site is 14C and is approximately 2.1 ac with an estimated cost of \$1,784,550. This SMF is located at Sta. 1580+00 (RT) north of Moody Lake. Although the 100-year Floodplain abuts I-75 in this basin, measures will be implemented in the design phase to minimize any impacts to the 100-year floodplain along Moody Lake; therefore, Basin 14 does not require floodplain compensation. Based on proximity to outfall, and lowest cost, the preferred alternative for Basin 14 is SMF Site 14C.

<u>BASIN 15</u>

The preferred SMF site is 15A and is approximately 7.8 ac with an estimated cost of \$1,496,845. This SMF is located at Sta. 1595+00 (RT) south of Blanton Rd. Basin 15 does not require floodplain compensation. Based on proximity to outfall, and lowest cost, the preferred alternative for Basin 15 is SMF Site 15A.

BASIN 16

The preferred SMF site is 16A and is approximately 7.9 ac with an estimated cost of \$1,044,300. This SMF is located at Sta. 1612+00 (LT) in the northwest quadrant of Blanton Rd. and I-75 and a portion of it is in the northeast infield of this interchange. Basin 16 does not require floodplain compensation. Based on proximity to outfall, and lowest cost, the preferred alternative for Basin 16 is SMF Site 16A.

BASIN 17

The preferred SMF site is 17B and is approximately 8.7 ac with an estimated cost of \$1,087,550. This SMF is located at Sta. 1691+00 (RT) north of Mud Lake and west of McClendon Lake. Although the 100-year Floodplain abuts I-75 in this basin, the existing ground elevations within the right-of-way are above the estimated 100-year floodplain; therefore, Basin 17 does not require floodplain compensation. Based on proximity to outfall, and lowest cost, the preferred alternative for Basin 17 is SMF Site 17B.

BASIN 18

The preferred SMF site is 18B and is approximately 7.5 ac with an estimated cost of \$2,919,850. This SMF is located at Sta. 1707+00 (RT) north of McClendon Lake. The preferred alternative does impact the 100-year Floodplain and requires floodplain compensation. Floodplain compensation can be accomplished using a portion of SMF 18A that isn't within the 100-year floodplain. The size of the FPC is estimated to be 3.0 ac and cost \$470,500. Based on proximity to outfall, and lowest cost, the preferred alternative for Basin 18 is SMF Site 18B.

<u>BASIN 19</u>

The preferred SMF site is 19B and is approximately 7.8 ac with an estimated cost of \$179,500. This SMF is located at Sta. 1763+00 (LT) near Cross Drain No. 21. The preferred alternative impacts the forest; however it is part of a parcel that isn't currently being managed by the Division of Forestry. Based on proximity to outfall, and lowest cost, and coordination with the Division of Forestry, the preferred alternative for Basin 19 is SMF Site 19B.

<u>BASIN 20</u>

The preferred SMF site is 20B and is approximately 4.7 ac with an estimated cost of \$885,470. This SMF is located at Sta. 1794+00 (LT) near Cross Drain No. 24. The preferred alternative does not impact the 100-year Floodplain. Based on proximity to outfall, and lowest cost, the preferred alternative for Basin 20 is SMF Site 20B.

<u>BASIN 21</u>

The preferred SMF site is 21B and is approximately 6.7 ac with an estimated cost of \$995,000. This SMF is located at Sta. 1825+00 (RT) near Cross Drain No. 27. The preferred alternative does not impact the 100-year Floodplain. Based on proximity to outfall, and lowest cost, the preferred alternative for Basin 21 is SMF Site 21B.

BASIN 22

The preferred SMF site is 22A and is approximately 7.1 ac with an estimated cost of \$1,164,800. This SMF is located at Sta. 1862+00 (LT) near Cross Drain No. 28. The preferred alternative does not impact the 100-year Floodplain. Based on proximity to outfall, and lowest cost, the preferred alternative for Basin 22 is SMF Site 22A.

BASIN 23

The preferred SMF site is 23A and is approximately 4.8 ac with an estimated cost of \$741,600. This SMF is located at Sta. 1896+00 (RT) near Cross Drain No. 30. The preferred alternative does not impact the 100-year Floodplain. Based on proximity to outfall, and lowest cost, the preferred alternative for Basin 23 is SMF Site 23A.

BASIN 24

The preferred SMF site is 24B and is approximately 7.1 ac with an estimated cost of \$1,051,400. This SMF is located at Sta. 1933+00 (LT) near Cross Drain No. 33. The preferred alternative does not impact the 100-year Floodplain. Based on proximity to outfall, and lowest cost, the preferred alternative for Basin 24 is SMF Site 24B.

BASIN 25

The preferred SMF site is 25C and is approximately 5.1 ac with an estimated cost of \$766,200. This SMF is located at Sta. 1987+00 (LT) near Cross Drain No. 35. The preferred alternative does not impact the 100-year Floodplain. Based on proximity to outfall, and lowest cost, the preferred alternative for Basin 25 is SMF Site 25C.

BASIN 26

The preferred SMF site is 26C and is approximately 5.2 ac with an estimated cost of \$789,400. This SMF is located at Sta. 2006+00 (RT) near Cross Drain Nos. 36 and 37. The preferred alternative does not impact the 100-year Floodplain. Based on proximity to outfall, and lowest cost, the preferred alternative for Basin 26 is SMF Site 26C.

<u>BASIN 27</u>

The preferred SMF site is 27C and is approximately 3.7 ac with an estimated cost of \$588,900. This SMF is located at Sta. 2028+00 (RT) near Cross Drain No. 39 and south of US 98/SR 50. The preferred alternative does not impact the 100-year Floodplain. Based on proximity to outfall, and lowest cost, the preferred alternative for Basin 27 is SMF Site 27C.

BASIN 28 Not used.

BASIN 29

Since the least expensive SMF alternative is within the forestry property, this alternative was not selected as the preferred alternative. It is the opinion of the Department's PD&E staff through prior coordination with the Division of Forestry that excavated SMF alternatives within state forest lands will not be permitted by the Division of State Lands. Actual cost of

acquisitions and/or easements within forestry lands will be subject to a negotiated mitigation package. Since these costs may not be indicative of the final mitigation costs; this criterion was not considered a critical factor in the selection of the preferred SMF alternative. Therefore, the preferred SMF site is 29C. SMF 29C is approximately 7.6 ac with an estimated cost of \$3,287,700. 29C is located at Sta. 2068+00 (RT) and does not impact the 100-year floodplain. Based on proximity to the outfall and avoiding an impact to a 4(f) property, the preferred alternative for Basin 29 is SMF Site 29C.

<u>BASIN 30</u>

There are four alternatives within Basin 30; two of which involve a natural depression area named 3a/3bC in combination with a SMF site. The natural depression area is located on forestry property. SMF Alternatives 30A and 30B are not located on forestry property and are avoidance alternatives. Based on proximity to outfall, and lowest cost, the preferred alternative for Basin 30 is SMF Site 30B. SMF Site 30B is approximately 15.0 ac with an estimated cost of \$7,669,280. This SMF is located at Sta. 2162+00 (RT) near Cross Drain No. 45.

BASIN 31

There are four alternatives within Basin 31; two of which involve a natural depression areas located on the forestry property. SMF 31A and 31B are also located on the forestry property. The only alternative not on the forestry property is SMF 31D in combination with two natural depression areas. In order to minimize any impact to the forestry property, the preferred alternative for this basin is the alternative with SMF 31D in combination with two natural depression areas, 4b(e)C and 4b(w)C. SMF 31D is located at Sta. 2200+00, (RT) and the two natural depression areas, 4b(e)C and 4b(w)C, are located at Sta. 2233+00 (RT) and 2233+00 (LT), respectively. The total area for SMF 31D and the natural depressions is approximately 13.9 acres. The total area for SMF 31D and Perpetual Transportation/Drainage/Maintenance Easements within the Forest is 20.4 acres.

It is the opinion of the Department's PD&E staff through prior coordination with the Division of Forestry that excavated SMF alternatives within state forest lands will not be permitted by the Division of State Lands. Actual cost of acquisitions and/or easements within forestry lands will be subject to a negotiated mitigation package. Since these costs may not be indicative of the final mitigation costs; this criterion was not considered a critical factor in the selection of the preferred SMF alternative.

BASIN 32

There are three alternatives within Basin 32; one of which involves two natural depression areas located on the forestry property. SMF 32A and 32B are also located on the forestry property. The preferred alternative for this basin is the alternative with two natural depression areas and does not contain a SMF site. These two natural depression areas are named 5aC and 5bC and are located at Sta. 2252+00 (LT), 2265+00 (LT), respectively. The estimated impacted area for the natural depressions is approximately 12.0 acres. The total area for Perpetual Transportation/Drainage/Maintenance Easements that would encompass the impacted area and areas of stormwater conveyance within the Forest is 53.1 acres.

It is the opinion of the Department's PD&E staff through prior coordination with the Division of Forestry that excavated SMF alternatives within state forest lands will not be permitted by

the Division of State Lands. Actual cost of acquisitions and/or easements within forestry lands will be subject to a negotiated mitigation package. Since these costs may not be indicative of the final mitigation costs; this criterion was not considered a critical factor in the selection of the preferred SMF alternative.

BASIN 33

There are three alternatives within Basin 33; one of which involves two natural depression areas located on the forestry property. SMF 33A and 33B are also located on the forestry property. The preferred alternative for this basin is the alternative with two natural depression areas and does not contain a SMF site. These two natural depression areas are named 6a/bC and 6cC and are located at Sta. 2300+00 (RT), 2339+00 (RT), respectively. The estimated impacted area for the natural depressions is approximately 9.4 acres. The total area for Perpetual Transportation/Drainage/Maintenance Easements that would encompass the impacted area and areas of stormwater conveyance within the Forest is 37.3 acres.

It is the opinion of the Department's PD&E staff through prior coordination with the Division of Forestry that excavated SMF alternatives within state forest lands will not be permitted by the Division of State Lands. Actual cost of acquisitions and/or easements within forestry lands will be subject to a negotiated mitigation package. Since these costs may not be indicative of the final mitigation costs; this criterion was not considered a critical factor in the selection of the preferred SMF alternative.

<u>BASIN 34</u>

There are three alternatives within Basin 34; one of which involves a natural depression area located on the forestry property. SMF 34A and 34B are also located on the forestry The preferred alternative for this basin is the alternative with the natural property. depression area and does not contain a SMF site. This natural depression area is named 7C and is located at Sta. 2345+00 (LT). The estimated impacted area for the natural depression is approximately 7.4 acres. The total area for Perpetual Transportation/Drainage/Maintenance Easements that would encompass the impacted area and areas of stormwater conveyance within the Forest is 10.8 acres.

It is the opinion of the Department's PD&E staff through prior coordination with the Division of Forestry that excavated SMF alternatives within state forest lands will not be permitted by the Division of State Lands. Actual cost of acquisitions and/or easements within forestry lands will be subject to a negotiated mitigation package. Since these costs may not be indicative of the final mitigation costs; this criterion was not considered a critical factor in the selection of the preferred SMF alternative.

Appendix A Concept Plans



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Appendix B Correspondence

Record of Telephone Call

H. W. LOCHNER, INC., 13577 Feather Sound Drive, Suite 600, Clearwater, FL 33762

То	John Kenty; File - 1948	Date	February 9, 2006
From	Shelly Saunders		
CC	Rick Sowers		
Subject Spoke	I-75 PD&E Study; FPN 411014-1-22-01; FAP 0751-1201 FDOT Maintenance Coordination for Sumter County Randy Prescott – EDOT Leesburg		
with	Maintenance Office		
Phone No.	(352) 315-3100		

I called Mr. Randy Prescott with the Leesburg FDOT maintenance office to inquire about historical flooding issues along the subject project area. Our discussion is summarized below:

In general, his experience has been with the Sumter portion of our Interstate 75 Study. He stated that to his knowledge and understanding of previous complaints, there are no historical problems with flooding and there has been no overtopping of I-75.

I told Mr. Boone we will consider the issues we discussed in the study process.



To Herschel Conner; File - 1948

Date September 19, 2005

From Rick Sowers

cc Panos Kontses, Oscar Auler

Subject I-75 PD&E Study; FPN 411014-1-22-01; FAP 0751-120I FDOT Maintenance Coordination
 Spoke Larry Boone – FDOT Brooksville Maintenance Office with 16411 Spring Hill Drive; Brooksville, FL 34604

Phone (352) 797-5700

No.

I called Mr. Larry Boone with the Brooksville FDOT maintenance office to ask about his experience with the "natural conveyance and storage areas" used for stormwater management on two sub-basins for the US 98 improvements in Hernando and Citrus County near the Suncoast Parkway, and to inquire about historical flooding issues along the subject project area. Our discussion is summarized below:

In general, his experience has been with the Hernando and Pasco Counties' portion of our Interstate 75 Study. He stated that to his knowledge and understanding of previous complaints, there are no historical problems with flooding and there has been no overtopping of I-75. He also noted that the Hernando/Pasco portion of I-75 is under asset management through Infrastructure Corporation of America (ICA).

To his knowledge, he also stated that their have been no maintenance issues up to this point, since the completion of improvements to US 98, including the 2 sub-basins that utilize natural conveyance to existing storage areas. In his opinion, and in consideration of the general soils characteristics, particular attention should be given to conditions south of the first overpass located south of SR 50, where soils conditions may not be as favorable.

I told Mr. Boone we will consider the issues we discussed in the study process.

LOCHNER

H. W. LOCHNER, INC., 13577 FEATHER SOUND DRIVE, SUITE 600, CLEARWATER, FLORIDA 33762

(727) 572-7111 FAX (727) 571-3371

I-75 PD&E Study from SR 52 in Pasco County to South of CR 476 in Sumter County FPN: 411014-1-22-01; FAP: 0751-120I

Meeting Minutes – SWFWMD Pre-Application Meeting Meeting Date: March 15, 2005

The following summarizes notes taken by Angie Patterson and Rick Sowers at the abovereferenced meeting. The purpose of the meeting was to introduce the project and establish criteria for the development of stormwater management facility (SMF) recommendations for the widening of I-75 from 4 lanes to the proposed typical section in Pasco, Hernando and Sumter Counties.

- Attendees: Wojeich Mroz, SWFWMD Kim Dorsten, SWFWMD Tim Polk, PBS&J/FDOT Rick Sowers, H.W. Lochner, Inc. Angie Patterson, H.W. Lochner, Inc.
 - <u>Basin Studies</u>: No studies have taken place in Sumter county; however, there are a number of basin studies that have taken place along this project in Pasco and Hernando Counties, including:
 - Withlacoochee River Study (from SWFWMD)
 - SR 52 area/Cypress Creek/Bella Verde (South of SR 52)
 Contacts: Mike Finch at RS&H, David Arnold of SWFWMD
 - o Old Pasco Road (by Pasco County/King Engineering)
 - o Hernando County Studies, check with Gene Altman of SWFWMD
 - Published Study of Lake Levels (SWFWMD)
 - <u>Water Quality Treatment Criteria</u>: in the case where we must do re-construction (change of profile, etc.), we must treat all of the directly connected impervious area (DCIA); i.e. all lanes plus shoulders. In the case, however, where we are widening only, we must treat a <u>minimum</u> of the new impervious area (added lanes) and we are <u>strongly encouraged</u> to treat as much DCIA as we can possibly treat. It was noted that FDOT will likely support the treatment of all DCIA, where possible.

- Treatment Criteria for Withlacoochee River Basin: this basin will require 50% more treatment for direct discharge to the river since it is an Outstanding Florida Water (OFW).
- Treatment Criteria for basins with proposed discharge to active sinkholes: double treatment will be required where discharge is to basins that are determined to have active sinkholes according to a geotechnical evaluation.
- <u>Water Quantity Criteria</u>: It was noted that there will probably be some open and some closed basins along this project. We received graphics that show the design of wet detention facilities (conservation method) in both open and closed basins. See attached graphics.
- <u>TMDL Criteria</u>: When asked what the chances are that we will have to deal with TMDL criteria for this project, The SWFWMD's response indicated that the application of TMDL criteria is still a long way off and the time frame is unknown at this time. It is appropriate to use presumptive criteria for this project.
- <u>Linear SMF's</u>: Although not preferred from a safety standpoint, if we should choose to use linear ponds within existing right-of-way (in the vicinity of potential Section 4(f) resources), SWFWMD's only comment is that we should use 4:1 side slopes and a depth of only 1-2 ft. for safety reasons.
- <u>SMF's in the Withlacoochee Forest</u>: Tim Polk posed the idea of not implementing SMF's in the area through the Withlacoochee Forest where there are closed depressions off-site. Instead, he suggested allowing the runoff to naturally flow to the low point of the depression. Wojeich agreed that this would be a viable alternative with appropriate modeling of the proposed condition and agreement of the property owner. The key will be to see if the Division of Forestry is interested in pursuing this option.
- <u>Sovereign Submerged Lands</u>: We need to check if the Withlacoochee River is a sovereign submerged land. If so, proprietary authorization may be needed from the state.

THIS SPACE IS FORMATTED TO FACILITATE AND GUIDE THE DIALOGUE DURING A PRE-APPLICATION MEETING AND PROVIDE NOTE TAKING SPACE. A SUPPLEMENTAL "PROMPT LIST" OF DISCUSSION ITEMS IS ATTACHED, WHICH SHOULD BE EXAMINED BY THE APPLICANT PARTIES PRIOR TO THE MEETING TO IDENTIFY TOPICS FOR DISCUSSION. Southwest Florida Water Management District FILE No. **Resource Regulation Division ERP Pre-Application Meeting NOTES** 3/15/15 Date: Time: 1-75 IN CASED, OFFRANDO, DU MTER ANGIE PATTERSON (727) 572 7111 Project Name: Attendees: S/T/R: 2.9/ 22/2/ Project acreage: 2.0 with s County: PSAD, HERAMADD, SULTER Total Land acreage: 20 miles Prior Onsite/Offsite Permit activity: 4 LAWAS HIGHLAN G ELISTING Project Overview: Site Information Discussion: (Site Topography, SHW Levels, Flood plain Elevations, Conveyance and Storage, Tailwater Conditions, Adjacent Offsite Contributing Sources, Receiving Waterbody, Karst Formations, Existing Wells, Contaminated Sites / Coordination w/ FDEP, etc.) Environmental Discussion: (Wetlands Onsite, Wetlands On Adjacent Properties, Site Visit, Delineation, Permanent/Temporary Impacts, SHWL, Wetland Hydrology, Drawdown Issues, Alternatives Analysis, Elimination/Reduction, Secondary and Cumulative Impacts, T&E species, Conservation Easements, Buffers, Mitigation Options, Mitigation Costs, OFW, Aquatic Preserve, stee stade of 20. w. and maybe whin R.O. w. T mitigation plan. etc.) 100 Sovereign Lands Discussion: (Title Determination, Delegated Authority, Correct Form of Authorization, Content of Application, Assessment of Fees, Coordination with FDEP, etc.) Widening over With scorches R. Needs Will Petermination Water Quantity Discussion: (Basin Description, Design Storm Event, Pre/Post Volume, Pre/Post Discharge, Local Requirements, Other) ADI TOLM TRAFFIC LAWE - OPEN & CLUSED BASIN, OENA ATTIMA ONVE ARNOUD TO BA COLORED FOR ADD TOBES Water Quality Discussion: (Type of Stormwater Treatment, Technical Characteristics, Non-presumptive Alternatives, Construction Phase Water Management and Erosion Control, Contaminated Sites, Ground Water Protection, etc.) NEW TO PUWH KS HUCH AS COSSIVERM OF WATER OC FEFISITAT PAVENEN

OPERATIONAL ERP Pre-Application Meeting NOTES

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41.00-107 (09/00)

Operation And Maintenance, Legal Information: (Ownership or Perpetual Control, Eminent Domain, Work on District Property, Inspections During Const., O&M Entity, System O&M Instructions, Homeowner Association Documents, Coastal Zone Requirements, Public Safety, etc.) Application Type And Fee Required: (40D-4.041Permits Required, 40D-1.607 Fee Schedule, etc.) FRIDD. 00, WETLAND MATAS ONCIACKE 1ADIVIAUN Other: (Future Pre-Application Meetings, Fast Track, Submittal Date, Construction Start Date, Required District Permits - WUP, WOD, Well Construction, etc.) Disclosure: The District ERP pre-application meeting process is a service made available to the public to assist interested parties in preparing for submittal of a complete permit application. Information shared at pre-application meetings is superseded by the actual permit application submittal. District permit decisions are based upon information submitted during the application process and Rules In effect at the time the application is complete. The following person was present and authored these ERP Pre-Application Meeting NOTES on behalf of the SOUTHWEST FLORIDA WATER MANAGEMENT DISTRICT: **District Staff Representative** Name and Title 15 05 laned Date



MANMADE WET DETENTION IN AN OPEN BASIN CONSERVATION DESIGN



MANMADE WET DETENTION IN A CLOSED BASIN

CONSERVATION DESIGN

Appendix C SMF Sizing and Estimated 100-year Floodplain Impact Calculations

SMF Sizing Calculations

69	rainage Area (ac)
(ac) A E	(ac) A E
6.12 0% 3	24.3 6.12 0% 3
6.63 0%	26.4 6.63 0% 1
5.67 0%	22.3 5.67 0%
5.30 0%	20.9 5.30 0%
2.13 0%	8.4 2.13 0%
3.21 0%	12.6 3.21 0%
3.56 0%	76.7 7.7 0%
0.11 0%	10 0.11 0%
2.32 0%	9.5 2.32 0.%
7.42 0%	32.6 7.42 0%
2.29 0%	9.2 2.29 0%
5.90 50%	29.0 5.90 50%
6.69 80%	39.1 6.69 80%
8.83 20%	39.2 8.83 20%
1.14 25% A70/	47 F 8 84 47 10 10 10 10 10 10 10 10 10 10 10 10 10
3.63 25%	24.5 3.63 25%
7.61 45%	34.2 7.61 45%
7.55 100%	29.9 7.55 100%
5.53 100%	21.7 5.53 100%
8.06 100%	33.1 8.06 100%
4.01 1007 A 73 1006	50.4 4.01 1007
7.19 100	23.4 7.19 100
2.46 100	13.2 2.46 100
1.80 100	7.2 1.80 100
3.51 1009	14.1 3.51 100
2.47 1007 2.44 1009	9.1 0.42 1007 0.1 0.44 1007
4.20 100	16.6 4.20 100
6.49 100%	28.3 6.49 100%
6.11 100%	21.3 6.11 100%
6.11 100%	21.3 6.11 100%
2.97 100%	13.1 2.97 100%
2.97 100%	13.1 2.97 100%
2.11 100%	8.2 2.11 100%
3.63 100%	14.8 3.63 100%
3.07 0%	10.4 3.07 0%
5.91 60%	41.2 5.91 60%
1001 62.2	100 100 100
00 0.21	00 10.0 100

	Basin	Length	Impervious	Drainage	Impervious	% Hyd	rologic Sc	oil Group	(HSG)	%	т	SG - % Ir	nperviou	s	I	SG - % P	ervious		
		(ii)	Width (ft)	Area (ac)	(ac)	A	۵	υ	D	Impervious	A	8	U	0	٨	в	υ	<u>د بر</u>	R55 Results
	2	3510	216	24.3	17.40	%0	35%	%09	5%	72%	%0	25%	43%	4%	%0	10%	17%	1%	92
	<u>،</u> ا	7090	510	18.6	13.34	%0	%0	80%	20%	72%	%0	%0	57%	14%	%0	%0	23%	6%	93
	1 u	3250	210	4.02	18.84	%0	15%	85%	%0	71%	%0	11%	61%	%0	%0	4%	24%	%0	92
	2	3040	216	0.02	10.12	%0	%0	100%	%0 2007	72%	%0	%0	72%	%0	%0	%0	28%	%0	93
	2	1220	216	8.4 8.4	6.05	200/	50%	30%	10%	0/2/	%0	%0	22%	50%	%0	%0	8%	20%	94
	8	1840	216	12.6	9.12	%0	100%	0%	0/00	0/.71	%n	0/00	%0	30%	%0	14%	%0	14%	92
	6	2040	216	14.1	10.12	%0	35%	65%	%0	2/0/21	%0	750	170/	%0	%0	700/	100/	0%0	06
	10	3880	216	28.2	19.24	0%0	55%	10%	35%	68%	200	370%	707	1010	%n	1070	0/01	0%0	76
	11	1320	216	10.9	6.55	%0	45%	55%	%0	60%	%0	2/02/0	1/022	0/ 470 /0/	2/0/	1070	0/0		19
	12	1330	216	9.5	6.60	%0	65%	35%	0%	20%	%0	45%	7070	200	200	0/01	148/	200/	800
	13	4250	216	32.6	21.07	%0	%0	80%	10%	65%	%0	0%	58%	8%	%0	0%0	2/065	0/D	01 01
	14	1310	216	9.2	6.50	0%	0%0	75%	25%	71%	%0	%0	53%	18%	%0	%0	22%	70/2	10
	15	1900	216	29.0	16.55	50%	%0	50%	%0	57%	29%	%0	29%	%0	22%	0%	22%	%0	84
	16	3600	216	39.1	25.05	80%	%0	20%	0%	64%	51%	%0	13%	%0	29%	0%	7%	200	82
	71	5060	216	39.2	25.09	20%	10%	20%	50%	64%	13%	6%	13%	32%	7%	4%	7%	18%	06
100 4	8	4090	216	36.3	20.28	25%	%0	65%	10%	56%	14%	%0	36%	6%	11%	0%0	29%	4%	87
4(1)-1-1	200	0000	216	47.5	25.04	47%	0%0	53%	0%	53%	25%	%0	28%	0%0	22%	0%	25%	0%0	82
	20	2080	216	24.5	10.31	25%	%0	75%	%0	42%	11%	0%	32%	0%0	14%	0%	43%	0%0	83
	5	4360	216	34.2	21.62	45%	%0	55%	%0	63%	28%	0%0	35%	0%0	17%	0%	20%	0%0	86
	72	4320	917	29.9	21.45	100%	%0	%0	%0	72%	72%	%0	%0	%0	28%	0%0	%0	0%0	84
	52	0/10	917	7.12	15.72	100%	%0	%0	%0	72%	72%	0%	%0	%0	28%	0%	0%0	0%0	84
	24	1204	012	10.0	10.22	100%	%0	%0	%0	69%	69%	%0	%0	%0	31%	%0	%0	0%0	83
	200	0710	210	10.2	13.08	100%	%0	%0	%0	72%	72%	%0	%0	%0	28%	%0	%0	0%	84
	202	2750	240	1.00	13.45	100%	%0	%0	%0	23%	23%	%0	%0	%0	77%	0%	0%	0%	60
4(f)-2a	202	1410	210	40.4	13.04	100%	%0	%0	%0	58%	58%	%0	%0	%0	42%	%0	0%	0%	77
4(f)-2b	29h	1030	216	10.2	0.44	100%	%0	0%0	%0	53%	53%	%0	%0	%0	47%	%0	%0	0%0	48
4(f)-2c	290	2010	216	14.1	0.07	10007	0/00	0/00	%0	11%	/1%	%0	%0	%0	29%	%0	%0	0%	84
4(f)-3a	30a	1960	216	13.4	9.72	100%	%0	0%0	0%	1070	730/	%0	%0	%0	30%	%0	%0	%0	83
4(f)-3b	30b	1400	216	9.1	6.94	100%	%0	0%0	%0	%2L	%C1	%0	200	0/0	0/.17	%0	%0	0%0	85
4(1)-3c	30c	2410	216	16.6	11.95	100%	%0	%0	%0	72%	72%	%0	%0	%0	2R0/	200	0/0	0/00	10
4(f)-3d	304	3720	216	28.3	18.45	100%	%0	0%0	%0	65%	65%	0%0	%0	%0	35%	%0	0%0	200	81
1)-4a(w)	31a(w)	3500	216	21.3	17.36	100%	%0	0%0	%0	81%	81%	%0	%0	%0	19%	0%0	0%0	%0	B8
()-4a(e)	31a(e)	3500	216	21.3	17.36	100%	%0	%0	%0	81%	81%	%0	%0	%0	19%	0%	%0	%0	B B
(31D(W)	1700	216	13.1	8.43	100%	%0	%0	0%	64%	64%	%0	%0	%0	36%	0%	0%0	%0	BU
19061	(a)010		216	13.1	8.43	100%	%0	%0	%0	64%	64%	%0	%0	0%0	36%	0%0	0%0	%0	80
- 1- 2d	870	0171	917	8.2	6.00	100%	%0	%0	%0	74%	74%	%0	%0	%0	26%	%0	0%	0%0	85
00-(1)+	320	2080	216	14.8	10.31	100%	%0	%0	%0	70%	70%	%0	%0	%0	30%	%0	0%	0%	83
111-04	100	1/01	017	10.4	8.73	%0	26%	20%	54%	84%	%0	22%	17%	45%	%0	4%	3%	9%	95
00-(1)+	020	3387	912	47.2	16.80	60%	%0	40%	%0	36%	21%	%0	14%	%0	39%	0%	26%	%0	74
111-00	24	0971	210	1.1	6.35	100%	%0	%0	%0	83%	83%	%0	%0	0%0	17%	%0	0%	0%	06
	54	7240	Q17	18.4	11.11	66%	%0	34%	%0	60%	40%	%0	21%	0%0	26%	. %0	13%	0%	83
		-												-					
initi ince	מן זוובורווס	IIAc. III.pc	I NIOUS AI 64 1	ncreases by	45% Irom F	RF -	-	_			_						-		

	"Q"	Pre-Devel	opment (Conditions													
			Total		S	heet Flow			Shal	low Con	centrated Flo	A		TR	55 Resul	i	
	Bacin	Proposed	Length:	Cfoot		2-yr, 24-hr	Difference in			Flow	Difference in			25-Yr	100-Yr		
		Area (ac)	Longest Path (ft)	Description	(ft)	Figure B-3	Elevation over length /#/	Slope (ft/ft)	Surface Description	Length (ft)	Elevation over length	Slope (ft/ft)	Tc (hrs)	Peak Outflow	Peak Outflow	23-Yr Runoff (in)	Runoff (in)
	2	24.3	2434	Smooth Surface	34	2	0.60	0000		0070	Ê	0,00		(cfs)	(cfs)	1	1
	ო	18.6	2434	Smooth Surface	34	o lo	0.69	0.020	Innaved	2400	87	210.0	0.38	154	198	6.7	8.6
	4	26.4	3534	Smooth Surface	34	сл	0.69	0.020	Unnaved	3500	t 0	200.0	1.44	11	98	1.7	5
	2	22.3	2034	Smooth Surface	34	5	0.69	0.020	Unpaved	2000	20	0.000	11.1	157	123	6.9	6.9
	9	20.9	1534	Smooth Surface	34	S	0.69	0.020	Unpaved	1500] σ.	0.006	10.0	155	106		20
	~ •	8.4	834	Smooth Surface	34	2	0.69	0.020	Unpaved	800	11	0.014	10.12	86	110	4. y	4.4 7
	20	12.6	1434	Smooth Surface	34	сı	0.69	0.020	Unpaved	1400	1.4	0.001	0.77	50	65	0.0 1	7.0
	2	14.1	1/34	Smooth Surface	34	2	0.69	0.020	Unpaved	1700	2.1	0.001	0.93	22	71	5 2	2 9
	2	2.82	3534	Smooth Surface	34	2	0.69	0.020	Unpaved	3500	51	0.015	0.5	156	201	67	8.0
	= ¢	0.9	1334	Smooth Surface	34	5	0.69	0.020	Unpaved	1300	20	0.015	0.19	06	116	6.4	83
	2 4	3.5	1334	Smooth Surface	34	ں م	0.69	0.020	Unpaved	1300	16.8	0.013	0.2	17	66	6.4	8.3
	14	0.20	1124	Smooth Surface	45	ŋ	0.69	0.020	Unpaved	3600	71.8	0.020	0.45	201	256	7.1	6
	4	200	1011	Smooth Surface	34	۰ ۱	0.69	0.020	Unpaved	1100	24	0.022	0.13	96	122	7.2	9.1
	4	30.1	1004	Smooth Surface	34	ה ו	0.69	0.020	Unpaved	1300	26	0.020	0.17	210	280	5.5	7.3
	17	30.2	4034	Smooth Surface	45	n ı	0.69	0.020	Unpaved	2600	54	0.021	0.32	170	239	4.3	6.0
	18	36.3	4004	Smooth Surface	40	n r	0.69	0.020	Unpaved	4000	88	0.022	0.47	220	284	6.6	8.5
4(f)-1	19	47.5	2000	Smooth Surface	40	n u	0.69	0.020	Unpaved	3200	90	0.009	0.59	170	222	6.2	8.1
	20	245	1534	Smooth Surface	40	0	0.69	0.020	Unpaved	2065	42	0.020	0.26	291	388	5.5	7.3
	54	6 72	280A	Smooth Surface	14	Ω.	0.69	0.020	Unpaved	1500	30	0.020	0.19	190	248	6.1	7.9
	22	20.00	2414	Smooth Surface	34	Ωι	0.69	0.020	Unpaved	2770	67.1	0.024	0.31	203	269	5.7	7.5
	33	217	1800	Smooth Surface	45	ה ו	0.69	0.020	Unpaved	2380	52.6	0.022	0.28	133	189	4.2	5.9
	24	33.1	5429	Smooth Surface	45	n u	0.69	0.020	Unpaved	1865	23	0.012	0.30	145	205	4.2	5.9
	25	18.2	2306	Smooth Surface	5	n u	60.0	0.020	Unpaved	2395	27	0.011	0.40	125	177	4.2	5.9
	26	59.4	2077	Smooth Surface	5	ה ע	0.09	020.0	Unpaved	2272	14	0.006	0.47	63	06	4.2	5.9
	27	23.4	1459	Smooth Surface	5	ט מ	0.09	020.0	Unpaved	2043	13	0.006	0.43	158	237	3.2	4.7
4(f)-2a	29a	13.2	1334	Smooth Surface	PE	o u	090	020.0	Unpaved	0007	13	0.009	0.25	121	168	4.6	6.3
4(f)-2b	29b	7.23	1234	Smooth Surface	34	о I.С.	0.60	0.020	Ulpaved	1200	4	0.003	0.36	47	68	3.9	5.4
4(f)-2c	29c	14.1	1534	Smooth Surface	34	0 40	0.00	0.020	Unpaved	1500	71	0.010	12.0	36	51	4.2	5.9
4(f)-3a	30a	13.4	1634	Smooth Surface	34	o uo	0.69	0.020	Unpaved	1600	25	120.0	0.19	47	104	4.2	5.9
4(f)-3b	30b	9.1	1134	Smooth Surface	34	S	0.69	0.020	Ilnnaved	1100	-	100.0	0.4t	2	200	b.4	0.0
4(f)-3c	300	16.6	2134	Smooth Surface	34	2	0.69	0.020	Unpaved	2100	18		020	44 63	70		0.0
4(1)-30	aud	28.3	1634	Smooth Surface	34	5	0.69	0.020	Unpaved	1200	2	0.006	20.24	31	150	7.4	10.0
4(T)-48(W)	31a(w)	21.3	2034	Smooth Surface	34	5	0.69	0.020	Unpaved	2000	11	0.006	0.45	81	114	- u - t	
4(1)-4a(e)	31a(e)	21.3	2034	Smooth Surface	34	5	0.69	0.020	Unpaved	2000	11	0.006	0.45	81	114		
4(1)-4D(W)	31D(W)	13.1	2434	Smooth Surface	34	5	0.69	0.020	Unpaved	2400	26	0.011	0 40	40	102		- 0
4(1)-4D(C)	31D(e)	13.1	2434	Smooth Surface	34	5	0.69	0.020	Unpaved	2400	26	0.011	0.40	49	202	4.7	0.0
419 54	976	2.5	834	Smooth Surface	34	5	0.69	0.020	Unpaved	800	18	0.023	0.10	20	11	4.3	0.0
116 Gu	020	14.8	1134	Smooth Surface	34	5	0.69	0.020	Unpaved	1100	18	0.016	0.45	23	75	4.7	0.0
4/1-6h	12h	10.4	2034	Smooth Surface	34	5	0.69	0.020	Unpaved	2000	1	0.004	0.55	52	67	6.3	8.2
4(f)-6c	330	7.14	4047	Smooth Surface	34	۰ م	0.69	0.020	Unpaved	2400	18	0.008	0.47	193	265	4.8	6.6
4(f)-7	34	18.4	1734	Smooth Surface	34	<u>م</u>	0.69	0.020	Unpaved	800	14	0.018	0.11	52	72	4.5	6.1
					ち	-	PO'D	0.020	Unpaved	1700	31	0.018	0.22	109	149	50	6.7

	"Q"	Post-Deve	lopment	Conditions (8-la	anes)			- 4										
			Totol		Sh	leet Flow				wolledg	Concentrate	d Elour			L L	0		
	Basin	Proposed	Length:	, ,		2-yr, 24-hr	Difference in			Low	Difference in		010		25-Yr	100-Yr	::	
		Area (ac)	Longest Path (ft)	bescription	(ft)	Figure B-3 (in)	Elevation over length (ft)	Slope (ft/ft)	Surface Description	(ft)	Elevation over length	Slope (ft/ft)	VELOCITY (FIG 3-1)	Tc (hrs) (Peak	Peak Outflow	Z5-Yr Runoff 1 (in)	00-Yr Runoff (in)
	2	24.3	2448	Smooth Surface	48	5	0.96	0.020	Paved	2400	28	0.010		100	(cts)	(cts)		
	ر	18.6	2448	Smooth Surface	48	5	0.96	0.020	Paved	2400	4	0.002		10.74	101	402	το α	10
	4	26.4	3548	Smooth Surface	48	5	0.96	0.020	Paved	3500	6	0.003		1 88 0	128	160 .	7.0	10.1
	0	22.3	2048	Smooth Surface	48	5	0.96	0.020	Paved	2000	22	0.011		0.27	201	250	0 6	101
		20.9	1548	Smooth Surface	48	5	0.96	0.020	Paved	1500	6	0.006		0.27	191	237	1.0	10.3
	- 0	10.4	040	Smooth Surface	48	5	0.96	0.020	Paved	800	11	0.014		0.1	107	133	200	10
		977	1448	Smooth Surface	48	5	0.96	0.020	Paved	1400	1.4	0.001		0.61	13	60	7.8	
	5	14.1	1748	Smooth Surface	48	5	0.96	0.020	Paved	1700	2.1	0.001		0.74	76	70	2α	10.0
	2	7.97	3548	Smooth Surface	48	ß	0.96	0.020	Paved	3500	51	0.015		0 4	206	758	20	
	= ;	9.01	1348	Smooth Surface	48	ъ	0.96	0.020	Paved	1300	20	0.015		0.15	116	146	2.7	0.0
	2	0.0	1348	Smooth Surface	48	2	0.96	0.020	Paved	1300	16.8	0.013		0 16	100	126	- 2	
	2	32.0	3648	Smooth Surface	48	S	0.96	0.020	Paved	3600	71.8	0.020		95.0	251	314	0.2	0.0
	4	9.2	1148	Smooth Surface	48	5	0.96	0.020	Paved	1100	24	0.022		111	115	112	0.0	n
	2	29.0	1348	Smooth Surface	48	Q	0.96	0.020	Paved	1300	26	0.020		0 13	208	CH Car	2.0	
	2	1.60	2648	Smooth Surface	48	5	0.96	0.020	Paved	2600	54	0.021		0.25	304	301		0.0
		39.2	4048	Smooth Surface	48	5	0.96	0.020	Paved	4000	88	0.022		BE U	280	36.3	0.0	
110 4	2	5.05	3248	Smooth Surface	48	ۍ	0.96	0.020	Paved	3200	30	0.009		147	230	201	0.7	0.0
+(1)+	20	41.5	2113	Smooth Surface	48 ·	5	0.96	0.020	Paved	2065	42	0.020		1.00	307	200	τ. α	1.0
	77	C.42	1548	Smooth Surface	48	5	0.96	0.020	Paved	1500	30	0.020		15	236	302	0.0	
	7	34.2	2818	Smooth Surface	48	5	0.96	0.020	Paved	2770	67.1	0.024		7 25	285	362	2.2	0.0
	22	29.9	2428	Smooth Surface	48	5	0.96	0.020	Paved	2380	52.6	0.022		23.0	250	318	0. r	000
	22	7.12	1913	Smooth Surface	48	a	0.96	0.020	Paved	1865	23	0.012		24	178	200		
	24	1.00	2443	Smooth Surface	48	5	0.96	0.020	Paved	2395	27	0.011		32	236	301	. 0 9	0.0
	27	10.2	2320	Smooth Surface	48	2	0.96	0.020	Paved	2272	14	0.006		0.38	122	155	7.1	00
	27	4.00	1470	Smooth Surface	48	ں م	0.96	0.020	Paved	2043	13	0.006		0.34	234	334	4.1	210
4(f)-2a	29,9	13.0	13/8	Smooth Surface	48	ω ı	0.96	0.020	Paved	1425	13	0.009		0.20	181	236	6.2	8.1
4(f)-2b	79h	7.03	avct	Smooth Surface	0 1 1 1	0	0.96	0.020	Paved	1300	4	0.003	0	0.29	84	111	6.0	7.8
4(f)-2c	29c	14.1	1548	Smooth Surface	40	<u>م</u>	0.96	0.020	Paved	1200	12	0.010	0	.17	68	86	7.1	9.0
4(f)-3a	30a	13.4	1648	Smooth Surface	40	0 4	0.90	0.020	Paved	1500	32	0.021	0	0.15	136	174	6.9	8.9
4(f)-3b	30b	9.1	1148	Smooth Surface	48	יי כ	0.00		Laved	1000	~ -	0.004		0.23	114	145	7.2	9.1
4(f)-3c	30c	16.6	2148	Smooth Surface	48	о <i>и</i>	900	0200	Daved	0010		0.006		0.16	92	116	7.4	9.4
4(f)-3d	30d	28.3	1248	Smooth Surface	48	о и	0.00	0200	Daved		8	0.009		02.0	124	158	7.1	9.0
4(f)-4a(w)	31a(w)	21.3	2048	Smooth Surface	48) (c	0.06	0000		0021		0.000		.23	224	288	6.7	8.6
4(f)-4a(e)	31a(e)	21.3	2048	Smooth Surface	48) <i>ц</i> ,	90.0	0200	Daved		=	0.000		.29	175	220	7.7	9.6
4(f)-4b(w)	31b(w)	13.1	2448	Smooth Surface	48	0 40	0.96	000	Poved			0.000		.29	175	220	7.7	9.6
4(f)-4b(e)	31b(e)	13.1	2448	Smooth Surface	48	о к с	0.96	0000	Poved	0000	07	110.0		.34	86	111	6.6	3.5
4(f)-5a	32a	8.2	848	Smooth Surface	48	, v.	0.06	000	Daved	000	07	110.0		.34	86	111	6.6	3.5
4(f)-5b	32b	14.8	1148	Smooth Surface	48	о и	2000	0000		000	18	620.0		.10	93	118	7.2	9.1
4(f)-6a	33a	10.4	2048	Smooth Surface		2 11	0.00	020.0	Paved	0011	18	0.016	0	.38	97	124	6.9	3.9
4(f)-6b	33b	47.2	2448	Smooth Surface	ar ar	ט ע ט	0.80	0.020	Рачеп	2000	7	0.004	<u> </u>	.29	94	116	8.4	0.4
4(f)-6c	33c	7.7	848	Smooth Surface	48		000	1020	Paved	2400	18	0.008		.34	275	362	5.8	7.7
4(f)-7	34	18.4	1748	Smooth Surface	48	י נ	0.30	070.0	Laved	800	14	0.018		.10	95	119	7.8	8.0
			2.		24	5	0.30	177.	Рауец	00/1	31 10	0.018	0	.20	160	204	6.9	8.9

	Estimat	ed Stor	rmwater	r Managerr	nent Fac	cility (SN	IF) Volume	3 Requir	ements ((8-lanes)										
				Attenuat	tion Volu	me				Treatme	ant Volume				25.Va	ar Attani	Intion Vol			
		Pre			Pre	Post	100-Yr Poet													Total
	Basin	100	100-Yr	Proposed Drainage	100-Yr	100-Yr	Pre	Bacio	SMF	Required	%	Impervious	Required	Pre 25.	Post 25-					Required
		Runof	Runoff	Area	Kunoff Volume	Volume V	Attenuation	Type	Type	Treatment	Drainage	Drainage	Volume	Year Peak	Year Peak	Actual	Adjusted	1	25-Year	Volume
		f (ft)	(IL)	(ac)	(ac-ft)	(ac-ft)	ft)		(were un y)	(ii)	Area	Area (ac)	(ac-ft)	Outflow,	(CES)	Q ₀ /Q	Q _o /Q _i	Adjusted	Volume	(ac-ft)
	-	0.68	0.76	3.9	2.6	2.9	0.3	Open	Wet		0.56	22	0.0	10 00	EG.	0.00	au			
	2	0.72	0.83	24.3	17.4	20.3	2.8	Open	Wet	-	0.72	17.4	1.5	154	203	0.76	NIA	A/N	3 10	0.0
	~ ~	0./5	0.84	18.6	13.9	15.6	1.7	Open	Wet	-	0.72	13.3	1-1	17	101	0.76	NIA	NIA	01.0	0,4 7
	4 4	0.75	0.83	26.4	19.6	22.0	2.4	Open	Wet	-	0.71	18.8	1.6	96	128	0.75	AIN	A/N	3.40	2.0
	- u	04.0	0.04	52.3	19.7	18.8	2:0	Open	Wet	F	0.72	16.1	1.3	157	201	0.78	N/A	N/A	2.80	41
	~	0.70	0.00	8.02	10.4	70.0	9.6	Open	Wet	-	0.72	15.1	1.3	155	191	0.81	0.8	193.75	2.50	3.8
		0.66	0.87	10.4	- 0 0	0.7	6.0 C	Open	Wet	-	0.72	6.0	0.5	86	107	0.80	0.8	107.5	1.00	1.5
	0	0.72	0.83	14.1	101	11.7	2.U	Open	Wet	-	0.72	9.1	0.8	50	73	0.68	N/A	N/A	1.70	2.5
	10	0.72	0.83	28.2	20.2	23.3	 	Open	Wet		0.72	10.1	0.8	55	76	0.72	N/A	N/A	1.90	2.7
	11	0.69	0.80	10.9	7.6	8.7	1.1	Chen	Wet		0.08	19.2	1.6	156	206	0.76	NIA	N/A	3.50	5.1
	12	0.69	0.82	9.5	6.5	7.7	10	Open	Wet		02.0	0.5	0.5	8	116	0.78	AIN	N/A	1.30	1.8
	13	0.75	0.83	32.6	24.4	26.9	2.4	Closed	Wat		0.10	0.0	0.0	11	100	11.0	N/A	NIA	1.20	1.7
	14	0.76	0.84	9.2	6.9	7.7	0.8	Closed	Wet		17.0	21.1 6.5	2.4	207	251	0.80	0.8	251.25	3.80	8.0
	15	0.61	0.75	29.0	17.7	21.8	4.1	Closed	Wet		0.57	19.0	0.0	00	CL1	0.83	0.8	120.00	1.10	2.4
	15mod	0.59	0.76	54.0	32.0	41.0	9.0	Closed	Wet		0.62	2.01	- c	017	298	0.70	NIA	NIA	3.50	9.0
	9	0.50	0.73	39.1	19.6	28.4	8.8	Closed			20.0	25.0	0.7	720	340	0.68	N/A	N/A	4.20	16.0
	17	0.71	0.82	39.2	27.8	32.0	4.2	Closed	Wet	-	0.64	25.1	1 1	0/1	304	0.56	AN	A/A	5.70	15.5
	18	0.68	0.78	36.3	24.5	28.4	3.9	Closed	Wet	-	0.56	20.3	1.1	170	602	0.70	NIA	N/A	4.80	11.1
4(1)-1	19	0.61	0.73	47.5	28.9	34.4	5.5	Closed	Wet	-	0.53	25.0	10	201	202	0.73	N/A	N/A	4.40	10.0
	8	0.66	0.74	24.5	16.1	18.2	2.0	Closed	Wet	-	0.42	10.3	- 00	100	195	1.13	A DO D	N/A	5.30	12.9
	21	0.63	0.78	34.2	21.4	26.5	5.1	Closed	Wet	-	0.63	21.6	1.8	203	285	0.71	N/A	0C.152	00.7	5.4
	22	0.49	0.75	29.9	14.7	22.4	7.7	Closed	Dry	0.5	0.72	21.4	0.9	133	250	0.53	N/A	A/N	4.70	12.1
	23	0.49	0.75	21.7	10.7	16.3	5.6	Closed	Dry	0.5	0.72	15.7	0.7	145	178	0.81	0.80	181 25	0.00	0.01 2 2 2
	24	0.49	0.74	33.1	16.3	24.6	8.3	Closed	Ъ	0.5	0.69	22.9	1.0	125	236	0.53	N/A	N/A	5 10	C.0
	22	0.20	0.75	18.2	8.9	13.6	4.7	Closed	ΡΩ	0.5	0.72	13.1	0.5	63	122	0.52	N/A	N/A	000	R 1
	27	0.53	0.40	19.40	23.3	28.2	4.9	Closed	2	0.5	0.23	13.4	0.6	158	234	0.68	N/A	N/A	4.40	0.0
4(f)-2a	29a	0.45	0.65	13.7	2.2	0.01	0.0	Closed	5	0.5	0.58	13.6	0.6	121	181	0.67	N/A	N/A	2.60	6.7
4(f)-2b	29b	0.49	0.75	7.2	3.6	2.4	1 0 1	Closed		0.0	0.53	7.0	0.3	47	84	0.56	N/A	N/A	1.70	4.6
4(f)-2c	29c	0.49	0.74	14.1	7.0	10.5	3.5	Closed		0.5	0.70	10.0	7.0	36	68	0.53	N/A	N/A	1.10	3.2
4(1)-3a	30a	0.50	0.76	13.4	6.7	10.2	3.5	Closed	Dry	0.5	0.73	9.7	0.4	t ç	114	40.0	A/M	N/A	2.10	6.1
4(1)-30	200	000	0./8	9.1	4.5	1.1	2.6	Closed	Dy	0.5	0.77	6.9	0.3	44	92	0.48	N/A	A/A	1 60	0.0
4(f)-3d	POR	84.0	0.70	0.01	8.1 7 7	12.4	4.3	Closed	Ъ С	0.5	0.72	12.0	0.5	63	124	0.51	N/A	N/A	2.70	7.5
4(f)-4a(w)	31a(w)	0.51	0.80	C 10	10.4	20.3	9.9	Closed	- Dry	0.5	0.65	18.4	0.8	111	224	0.50	N/A	N/A	4.40	12.0
4(f)-4a(e)	31a(a)	0.51	U BU	6 10	10.4	1.1.1	2.0	Closed	ĥ	0.5	0.81	17.4	0.7	81	175	0.46	N/A	N/A	4.00	10.9
4(f)-4b(w)	31b(w)	0.49	121	121	- Y 3	1.1	2.0	pesolo	λη ι	0.5	0.81	17.4	0.7	81	175	0.46	N/A	N/A	The state of the s	
4(f)-4b(e)	31b(e)	0.49	171	134	4.D	2.2	2.6	closed	- 20	0.5	0.64	8.4	0.4	49	86	0.57	N/A	N/A	1.80	5.0
4(f)-5a	32a	0.50	0.76	1.01 B.0	4 1	0.0 -	8.2	Desel	And .	0.5	0.64	8.4	0.4	49	86	0.57	NIA	NIA	The second second	Section of the local division of the local d
4(f)-5b	32b	0.49	0.74	14.8	1.5	11.0	2 1 2	losed	19/1	-	0.74	6.0	0.5	56	93	0.60	N/A	N/A	1.20	3.8
				2	2	2	0.1	Dason	Wel	-	0.70	10.3	6.0	53	97	0.55	N/A	N/A	2.20	6.8
4(f)-6a	33a	0.68	0.87	10.4	7.1	9.0	1.9	losed	Wat		10.0	1	F	4	-					
4(f)-6b	33b	0.55	0.64	47.2	26.0	30.3	4.3	Closed	Wat		1 3R	14.0	1.1	25	94	0.55	AIN	NIA	1.90	4.5
4(f)-6c	33c	0.51	0.82	7.7	3.9	6.3	2.4	Closed	Wet		0.00	0.0 A 2	4.I a c	193	275	0.70	NIA	N/A	4.80	10.5
4(f)-7	34	0.56	0.74	46.0	25.7	34.1	8.4	losed	Wat		0 BD	0.0	0.0	70	C27	0.55	N/A	NIA	1.30	4.2
									1044	-	0.00	21.5	2.3	109	160	0.68	N/A	N/A	2.30	13.0

	otal SMF Area roposed (ac)	2.7	2.7	2.8	2.2	2.2	2.2	4.0	4.1	4.0	5.0	4.6	4.3	3.1	3.1	3.1	1.5	1.7	1.5	2.2	2.2	2.2	2.2	2.5	2.4	4.1	4.1	4.1
-	atal Area Ic)	20	70	2	18	18	18	8	8	8	42	42	42	15	15	15	49	49	49	21	5	21	62	1	6,	6[60	6(
-	8 SMF (+1(+1)(a)(a)(a)(a)(a)(a)(a)(a)(a)(a)(a)(a)(a)	2	2.	N	N	N	~	4.(4.0	4.0	ы,	3.4	3.4	Э	ŝ	3.	1.4	1.4	1.4	2.2	2.2	2.2	1.1	2.4	1.7	4.0	4.0	4.0
	SMF SMF Clength (r Side Slopes Berms - 10%) (ff	457	457	457	408	408	408	565	565	565	520	520	520	498	498	498	334	334	334	413	413	413	368	433	368	571	571	571
	SMF Width (w/ Side Slopes & Berms + 10%) (ft)	257	257	257	233	552	233	309	309	309	286	286	286	275	275	275	194	194	194	233	233	233	212	243	212	312	312	312
	Total Req'd SMF Area (ac)	2.23	2.23	2.23	1.80	0.01	1.80	5.5	3.31	3.31	2.83	2.83	2.83	2.60	2.60	2.60	1.23	1.23	1.23	1.82	1.82	1.82	1.48	2.00	1.48	3.38	3.38	3.38
	SMF Length (w/ Side Slopes & Berms) (ft)	416	416	416	371	110	UP	210	212	513	473	473	473	452	452	452	304	304	304	375	375	375	334	394	334	519	519	519
	SMF Width (w/ Side Slopes & Berms) (ft)	234	234	234	212	717	717	107	107	1.07	700	260	260	250	250	250	176	176	176	212	212	212	193	221	193	284	284	284
	Width (ft)	20.00	20.00	20.00	20.00	00 00	20.00	20.00	00.02	70.00	20.00	20.00	20.00	20.00	20.00	20.00	20.00	20.00	20.00	20.00	20.00	20.00	20.00	20.00	20.00	20.00	20.00	20.00
	Water Surface Area @ Peak Design Stage (sq ft)	72759	72759	72759	200880	00000	120000	12000	10200	UDAUU	06006	95396	99256	86728	86728	86728	35896	35896	35896	8/2/9	8/2/9	57578	45052	53954	15052	16872	16872	16872
	SMF Length // Side Slopes (ft)	376	376	376	131	3	100	473	470	100	433	433	433	412	412	412	264	264	204	335	335	335	294	354	294 4	479 1	479 1	479 1
	SMF Width w/ Side Slopes (ft)	194	194	194	172	173	211	544	142	147	000	220	077	210	210	210	136	992	22	211	1/2	2/1	153	181	153	244	244	244
	Assumed Side Side Slope	4.00	4.00	4.00	4.00		4 00		DO V		00.4	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00
s)	SMF Length (ft)	364	364	304	310	310	ARF	ARE	ARE	425	101	420	C7+	404	404	404	907	007	007	170	170	32/	797	346	282	471	471	471
(8-lane	SMF Width (ft)	182	182	107	160	160	233	233	523	212	110	212	717	202	707	202	971	120	124	104	+0-	104	141	173	141	236	236	236
irements	Initial Required SMF Area (sq ft)	66072	66072	21000	20000	FUGRE	108252	108252	108252	0034	FCCUU	4020A	40700	01010	01010	01010	00/20	00/2C	22 LOU	20000		00000	07060	29/4Z	87865	091111	091111	111150
ea Requ	Initial Required SMF Area (ac)	1.52	1.52	117	1.17	117	2.49	2.49	2 49	207	2 07	207	1 00	00.1	00'	00.1	22.0	0.75	1 22	1 23	3 5	22.	10.0	15.1	1.61	20.7	20.7	CC.2
SMF) Ar	Required SMF Volume (ac-ft)	4.6	4.6	2 1	3.5	3.5	5.0	5.0	5.0	4.1	11	4.1			0.0	0.1		, u	240	240	, u	C.7	1.2	1.7	7.7			
Facility (Chosen SMF Depth (ft)	, m	n "	o e	0	6	0	2	2	2		40	10	10	4 0	4 0	4 0	40	• •	10	4 0	4 0	- - -	N (n (~ ~	~	7
anagement	Approximate Roadway Elevation (ft, NGVD)	98.8	90.0	0.00	94	94	91.4	91.4	91.4	105	105	105	118		118	115	115	115	111 1	111 1	111 1	4444	1111	1 1 1	111.1	114	+++	+ +
vater M	SCS Depth to CHWT (ft)		2.5-5	2.5-5	2.5-5	2.5-5	1.5-2.5	1.5-2.5	1.5-2.5	1.5-2.5	15-25	15-25		5	5	5	5 2		5		2	25.5	21		5.0	52	5 5	5
Ited Stormv	Est. Existing Ground Elevation of SMF Site (ft, NGVD)	000	96.5	87	87	87	86.5	83.5	84	105	100	102	105	108	106	108	105	103	105	107	105	105	103	110	100	110	801	001
Estima	SMF Alt.	AN RC	20	3A	38	g	4A	4B	4C	5A	58	50	6A	89	i iii	TA	ZB	70	BA	88	90	A6	æ	36	104	10H		202

- Г			Ú.	12/13	_	-	-	-	-	-				-	-			-		-					_	_	_	_			_					
			Total SMF	Proposed	(ac)		1.8	1.7	1.7	1.7	5.1	5.1	5.1	2.2	2.6	2.1	7.8	8.8	89	79	77	85	8.7	7.6	7.6	8.1	7.8		4.2	4.7	5.2	6.8	6.7	6.8	7.1	7.2
		Total	Req'd	SMF Area	(+110%) (ac)		1.75	1.49	1.67	1.67	5.04	5.04	5.04	2.17	2.17	1.84	6.76	6.76	6.76	7.80	7 80	8.20	8.20	7.45	7.45	7.72	7.72		4.30	4.30	4.30	6.81	6.81	6.81	6.79	6.79
		SMF	Length (w/ Side	Slopes &	Berms + 10%) (ft)		365	334	356	356	636	636	636	409	409	373	741	741	741	796	796	819	819	780	780	793	793		586	586	586	743	743	743	741	741
		SMF Width	(w/ Side	Slopes &	10%) (ft)		209	194	205	205	345	345	345	231	231	214	397	397	397	427	427	436	436	416	416	424	424		320	320	320	399	399	399	399	399
			lotal Ren'd	SMF Area	(ac)		1.44	1.23	1.38	1.38	4.17	4.17	4.17	1.79	1.79	1.52	5.58	5.58	5.58	6.45	6.45	6.77	6.77	6.16	6.16	6.38	6.38		3.55	3.55	3.55	5.63	5.63	5.63	5.61	5.61
		SMF	(w/ Side	Slopes &	Berms) (ft)	000	332	304	324	324	578	578	578	372	372	339	674	674	674	724	724	745	745	209	602	721	721		533	533	533	676	676	676	674	674
		SMF	(w/ Side	Slopes &	Berms) (ft)	001	3	111	186	186	314	314	314	210	210	195	361	361	361	388	388	396	396	378	378	386	386		290	290	290	363	363	363	363	363
			Berm		2	00.00	00.02	00.02	20.00	20.00	20.00	20.00	20.00	20.00	20.00	20.00	20.00	20.00	20.00	20.00	20.00	20.00	20.00	20.00	20.00	20.00	20.00	ľ	20.00	20.00	20.00	20.00	20.00	20.00	20.00	20.00
		Water Surface	Area @	Design	Stage (sn ft)	11 her	100004	14400	41403	4 483	14/3/1	141311	14/3/1	91505	56316	46337	203406	203406	203406	238023	238023	251045	251045	226320	026322	722200	235380	1	123489	123489	123489	205113	205113	205113	204607	204607
		SMF	Length	Slopes	(ŧ	000	707	102	107	204	000		000	332	332	RR7	634	634	634	684	684	705	705	699	600	8	081		493	493	493	636	636	636	634	034
		SMF	Width w/ Sido	Slopes	(¥)	150	137	146	140	041	574	174	170	110	21	8	125	175	321	348	348	356	356	995	338		340	010	007	002	220	323	323	323	525	323
			Assumed	Slope		4 NN	4 00	4 00		4 00	100	DO V	100	100	4.00	- no	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	100	4.00	100	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00
(Se			SMF	(H)		284	254	276	376	57B	578	528	VCE	170	1000	202	070	070	979	2/9	6/2	697	160	00	674	574	- 5	105	405	101	402	070	070	070	270	770
s (8-lane			SMF	æ		142	127	138	138	264	264	264	167	163	145	040	010	200	513	330	955	348	240		336	336	300	646	242	242	242	212	212	311	311	
irement:		Initial	Required	Area	(It ps)	40194	32155	38106	38106	13935A	139358	139358	52368	57368	41805	105030	105837	105020	200061	C0/C77	C0/C77	242622	240205	218326	275713	275213		1176 n G	117606	117606	105630	105630	105620	103272	103272	
ea Requ		Initial	Required	Area (ac)		0.92	0.74	0.87	0.87	3.20	3.20	3.20	1.20	1 20	960	A 50	4 50	A EO	4.00	. a	0.0	733	10.0	501	517	5 17		010	010	010	4 40	4 40	07 40	4 44	4.44	
SMF) Ar		Required	SMF	(ac-ft)		1.8	1.8	1.7	1.7	8.0	8.0	8.0	2.4	2.4	2.4	00	00	00	15.5	15.5	111		10.01	10.0	12.9	12.9		54	54	54	11.2	11 2	11 2	13.3	13.3	
Facility (Chasen	SMF	(iii)	:	2	2.5	2	~	2.5	2.5	2.5	2	2	2.5	6	10	10	4 (*			10	10	0	2.5	2.5		2	2	2	25	2.5	25	e	, m	
anagement		Approximate	Roadway	(ft, NGVD)		170	170	193	193	135	152	135	135	135	145	132.3	132.3	132.3	145	145	115.5	115.5	135.2	135.2	117	117		102.1	102.1	102.1	84.3	84.3	84.3	93.8	93.8	
vater M		SCS	9	SHWT	Ê	2	1.5-3.5	5		1.5-3.5	1.5-3.5	1.5-3.5	0-1	5	1.5-3.5	6-1	6-1-0	5	9<	9~	5-1	0-2.5	+2-1	+2-1	1.5-3.5	1.5-3.5		5	<u>-1-</u>	5	1.5-3.5	1.5-3.5	1.5-3.5	>6.0	>6.0	
ated Storm		Est. Existing Ground	Elevation of	A NGVDV	(11/0/1/11)	150	165	183	191	135	152	135	113	113	145	130	127	110	140	140	103	115	110	105	90	107	Vatural	83	81.5	60	17	80	11	87	90	
Estima	-		Alt Alt		4 4 4	ALL	118	12A	128	13A	138	13C	14A	14B	14C	15A	15B	15C	16A	16C	17A	178	18A	18B	19A	198	19C P	20A	20B	20C	21A	218	21C	22A	22B	
																									4(f)-1											

	Total SMF	Area Proposed (ac)	(an)	7.3	4.8	5.1	4.9	7.4	7.1	7.3	5.0	4.5	5.1	6.0	6.0	5.2	3.9	3.8	3.7	7.3	7.2	7.6	14.4	15.0	0	0.2	10.2	8.3	8.0	6.7	7.0	6.4	13.9	13.6	8.3	10.2
	Total	SMF Area (+10%)	(ac)	6.79	4.57	4.57	4.57	7.26	7.26	7.26	4.41	4.41	4.41	5.23	5.23	5.23	3.72	3.72	3.72	7.04	7.04	7.04	14.29	14.29	9.56	0 56	20.0	7.98	7.98	5.71	6.44	6.44	13.55	13.55	9.47	9.47
	SMF Length (w/	Side Slopes & Berms +	10%) (ft)	741	603	603	603	767	767	767	592	592	592	647	647	647	542	542	542	755	755	755	1087	1087	885	RR5	}	806	806	677	722	722	1060	1060	882	882
	SMF Width fw/ Side	Slopes & Berms +	10%) (Ħ)	399	330	330	330	412	412	412	325	325	325	352	352	352	299	299	299	406	406	406	572	572	471	471		431	431	367	389	389	557	557	467	467
	Total	Req'd SMF Area (ac)		5.61	3.77	3.77	3.77	6.00	6.00	6.00	3.65	3.65	3.00	4.32	4.32	4.32	3.08	3.08	3.08	5.82	5.82	5.82	11.81	11.81	7 00	22	7.90	6.59	6.59	4.72	5.33	5.33	11.20	11.20	7.82	7.82
	SMF Length	(w/ Side Slopes & Berms)	(H)	674	548	548	548	189	169	189	938	238	000	588	588	588	492	492	492	687	687	687	989	989	BOA		804	732	732	616	657	657	964	964	802	202
	SMF Width	(w/ Side Slopes & Berms)	(H)	363	300	300	300	C/E	9/P	5/5	CR7	282	067	320	320	320	212	272	272	369	369	369	520	520	428		428	392	392	334	353	353	506	506	425	675
	Bern	Width (ft)		20.00	20.00	20.00	20.00	20.00	20.00		20.00	20.02	00.00	20.00	20.00	20.00	20.00	20.00	20.00	20.00	20.00	20.00	20.00	20.00	20.00		20.00	20.00	20.00	20.00	20.00	20.00	20.00	20.00	20.00	20.00
	Water Surface Area @	Design	(th ps)	204607	132071	132021	132071	710010	102612	JCRR17	107005	197005	1000171	153034	400001	10205C1	/LOCOL	102017	102017	212868	212868	212868	455541	455541	296554	1 1 1 0 0 0	400067	243012	243812	1092/0	193191	LALEAL	430647	430647	R/7567	EITCRT
	SMF Length	w/ Side Slopes		634	208	800	508	100	100		100	108	000	040	040	240	704	452	452	647	647	647	949	949	764	102	104	760	720	0/0	/19	110	924	924	762	707
	SMF Width	w/ Side Slopes	1	323	260	007	200	200	200	345	255	255		107	007	700	707	232	232	329	329	329	480	480	388	000	257	252		234	515	212	400	100	385	200
	Assumed	Side Slope		4.00	4.00	4.00	4.00	1001	NOV	DO V		4 00	NON	100	nn'+	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	001	00 F	A 00 k	100 1	4.00	4.00	4.00	4 00	4 00 4	4.00	77.4
3S)	SMF	Length (ft)		622	490	400	645	RAF.	645	486	ARG	486	536	536			OFF V	1440	1440	550	650	635	100	831	752	753	1 de	ean Ban		100	202	100	010	754	754	5
s (8-lane	SMF	(li)		115	240	042	373	323	323	543	543	243	26A	268	200	220	220	022	177	115	115	115	400	400	376	376	OPE	An	Cac	302		AFB	458	377	377	
irements	Initial Required	SMF Area (so ft)		13261	123000	123000	208198	20810B	208198	118199	118199	118199	143855	143855	1A3REG	DEQA7	240AD	04047	14000	201303	201303	430540	130540	1420240	282871	282871	231423	231473	1580R7	183002	183007	410500	410500	284168	284168	
rea Requ	Initial Required	SMF Area (ac)		4.44	2.02	2 82	4.78	4.78	4.78	2.71	2.71	2.71	3.30	3.30	3 30	5 23	2 23	50.0	1 63	4.02 V	4.02	10.07	10.01	10.01	6.49	6 49	5.31	5.31	365	000	4 22	063	69.0	6.52	6.52	
SMF) A	Required SMF	Volume (ac-ft)	0.07	2.0	8.5	85	14.3	14.3	14.3	8.1	8.1	8.1	6.6	6.6	80	67	67	67	130	13.0	0.0	30.5	30.0	100	19.5	19.5	15.9	15.9	10.9	10.6	10.6	19.3	19.3	13.0	13.0	
Facility	Chosen SMF	(ft)	•	2 4			0	0	ص	9	e	e	m	9	6	6) er		,	9	ص	e	0	6	2.5	25	2	2	2	2	
anagement	Approximate Roadway	Elevation (ft, NGVD)	0.00	A7	87	87	83	83	83	96	96	96	86	86	86	74	74	74	73	73	2 5	78	78	2	78	78	73	73	76	65	65	59	59	62.5	62.5	
Valer M	SCS Depth	SHWT (fi)	- U Y	0.94	>6.0	>6.0	>6.0	>6.0	>6.0	>6.0	>6.0	>6.0	>6.0	>6.0 *	>6.0	>6.0	>6.0	>6.0	3.5-6	3.5-6	35.6	9<	9	-	9<	9<	-9	-94	9<	1.5-3.5	1.5-3.5	5		1.5-3	0-1	
	Est. Existing Ground	SMF Site (ft, NGVD)	85	85	80	85	75	20	82	95	95	95	81	82	85	65	65	65	70	202	65	20	20		20	70	68	65	20	58	57	55	48	55	60	
	SMF	Alt.	220	23A	23B	23C	24A	24B	24C	25A	258	25C	26A	26B	26C	27A	27B	27C	29A	298	290	30A	30B		30C	30D	31A	31B	31D	32A	32B	33A	33B	34A	34B	
																			4(f)-2			4(f)-3		4(f)-3c/	4(f)-3d	4(1)-3c/ 4(1)-3d	4(1)-4		4(f)-4a	4(f)-5		4(f)-6		4(f)-7		

Natural Storage Area Calculations

<u>Summary of Calculations for the Natural Storage Areas Located on 4(f)</u> <u>Property</u>

The areas of impact were calculated by estimating an initial and maximum stage elevation in each natural storage area and measuring the area for each respective elevation. The "initial" stage represents an estimate of the storage volume elevation for the existing (pre) I-75 condition. The "proposed" stage is an estimate of the storage volume elevation for the proposed (post) I-75 condition. Pre and post stages for the 100-year runoff volumes were then interpolated by calculating the available volume while ensuring that it is equal to or greater than the required storage volume. Once the pre and post stages were calculated, the post minus pre difference in elevations were determined. The maximum boundary elevations were used to establish the upper "impact" area and the initial boundary represents the lower "impact" area.

Given the uncertainties inherent to the locations of the contours shown on the SWFWMD aerial contour maps, the maximum boundary is shown at the upper limit of the depression to be conservative. The inside boundary of impact for each of the following natural storage areas are shown at the existing or pre stage. Therefore, the areas of impact were calculated by subtracting the area of the outer boundary from the area of the inner boundary. See the table below for the existing stage elevation, the proposed stage elevation and area of impact for each natural storage area.

Basin Name	Natural Storage Area Name	*Initial Boundary/ Existing Stage Elevation	*Proposed Stage Elevation	*Maximum Boundary Elevation	Estimated Area of Impact (ac)
30	3a/3bC	66.2	68.55	73	15.51
31	4a(w)C	56.7	57.02	62	6.65
31	4b(e)C	62.4	63.01	64	4.66
31	4b(w)C	62.6	63.44	66	2.52
32	5aC	56.6	56.97	59	6.81
32	5bC	49.3	49.75	54	5.23
33	6a/bC	50.1	50.6	53	3.67
33	6cC	55.7	56.02	60	5.67
34	7C	52.6	53.2	55	7.44

* All elevations in feet - NGVD '29

	TR55 Results:	Weighted CN	71	58	61	5	0	70	70	10	60	63	63	3 2	Lo	61	69	61	-	18	99	63	67
		ĺ	%0	%0	0%0	700	100	0/ 0	0100	0.20	%0	%0	700	200	0%0	%0	0%	%0	200	30%	0%	%0	0%0
ervious	U		43%	%0	%0	700	700	200	%n	20	%0	%0	70U	200	%0	%0	%0	%0	4 407	14%	35%	%0	27%
ISG - % I	m		%0	%0	%0	%0	760	200		225	%0	%0	%0	100	20	%0	0%	%0	100/	201	%0	%0	%0
-	A		38%	81%	75%	75%	70PL	705.2	750/	0/01	%//	71%	71%	1022	0/ 11	11%	74%	75%	100	0 ⁰ 0	9%7C	71%	52%
s	٥		%0	%0	%0	%0	%0	700	700	200	%0	%0	%0	700	200	%0	%0	%0	160/	200	%0	%0	%0
nperviou	ပ		10%	%0	%0	%0	%0	%0	700	200	%0	%0	%0	700	200	%0	%0	%0	Gu/L		%0	%0	7%
SG - % In	ш		%0	%0	%0	0%0	0%0	%0	7/00	20	%0	%0	%0	%0	100	%0	%0	%0	Ro/	200	%.0	%0	%0
Ŧ	A		9%	19%	25%	25%	26%	27%	25%	1000	0/07	%6Z	29%	23%	1000	0/.07	26%	25%	0%	100	0/0	29%	14%
%	Impervious	1001	19%	19%	25%	25%	26%	27%	25%	7307	0/07	79.40	29%	23%	730/	0/.07	26%	25%	29%	130/	0/01	0/,67	21%
(HSG)	۵	100	0%N	%0	%0	%0	%0	%0	%0	700	200	%D	%0	%0	700	200	%0	%0	54%	700		20	%0
oil Group	ပ	1002	03%	%0	%0	%0	%0	%0	%0	00/2	700	20	%0	%0	700	200	°%0	%0	20%	40%	20	200	34%
ologic Sc	ш	100	%.0	%0	%0	%0	%0	%0	%0	%0	/00	200	0%0	%0	760	200	<u>%</u> D	%0	26%	%0	/00	200	0%0
% Hydr	۷	10/V	P. 14	%nn1	100%	100%	100%	100%	100%	100%	100%	2001	%00L	100%	100%	10001	% On	100%	%0	60%	100%	2000	66%
Impervious Area	(ac)	8 81		2.40	1.80	3.51	3.42	2.44	4.20	6.49	6 11		0.1	2.97	2.97	2 11		3.63	3.07	5.91	2 23	201	3.91
Drainage	Area (ac)	47 F	0.04	2.6	1.2	14.1	13.4	9.1	16.6	28.3	213	0.10	0.12	13.1	13.1	8.7	4 4	14.8	10.4	47.2	77	101	10.4
Impervious		76	25	0	Q)	٩)	76	76	76	76	76	76	2	76	76	76	0.1	0,02	9	76	76	76	0
Length		5050	1410		0000	01.02	1960	1400	2410	3720	3500	3500	2000	1700	1700	1210	Couc	0007	1/00	3387	1280	2240	0477
 Basin		4(f)-1	4(P-2a	415 25	4(1)-20	11)-20	4(1)-38	4(t)-3b	4(f)-3c	4(f)-3d	4(f)-4a(w)	Aff-dala)	12/22 /2/2	4(1)-4b(w)	4(f)-4b(e)	4(f)-5a	ALFI EL	-9 (1)+	B0-(1)+	4(f)-6b	4(f)-6c	4(f)-7	

		TR55 Results:	Weighted CN	82	75	2	04	00	20	V0	50	0	89	89	80	80	85	83	95	74	06	83			
			כ	%0	0%	100	0/0	0/0	700	%0	100	%)	200	%n	%0	%0	%0	%0	9%	%0	%0	0%			
	Perviou		>	25%	0%	700	200	%0	0%	%0	700	0/0	200	0/0	0%0	%0	%0	%0	3%	26%	%0	13%			
		ď	נ	%0	%0	700	%0	%0	0%	%0	700	700	200	0/0	%0	%0	%0	%0	4%	%0	%0	%0			
	H		;	22%	47%	%006	30%	27%	23%	28%	35%	10%	1001	0/21	30%0	36%	26%	30%	%0	39%	17%	26%			
	sno	0	1	%0	0%0	0%0	%0	0%0	0%0	%0	%0	%0	100	200	%0	0%	%0	%0	45%	%0	%0	%0			
	npervic	0		28%	%0	%0	%0	0%0	%0	%0	%0	%0	700	200	0/0	%0	%0	%0	17%	14%	0%0	21%			
	1%-9	-	-	%0	%0	%0	%0	%0	%0	%0	%0	%0	700	200	0/0	%0	%0	%0	22%	0%0	%0	%0			
	SH	4		25%	53%	71%	70%	73%	77%	72%	65%	81%	810/	0/0/2	240	04%0	74%	%02	%0	21%	83%	40%			
	%	Impervious		53%	53%	71%	20%	73%	77%	72%	65%	81%	81%	EA02	104.0	04.70	74%	20%	84%	36%	83%	60%			
	(HSG)	٥		%0	%0	%0	%0	%0	%0	%0	%0	%0	%0	700	200	0.0	%0	%0	54%	%0	%0	%0			
Areas	ll Group	v		53%	0%0	%0	%0	0%0	%0	%0	%0	%0	%0	%0	700	0/0	%0	%0	0/.07	40%	%0	34%			
Storage	logic So	ш		%0	%0	%0	%0	%0	%0	%0	%0	%0	%0	%0	700	200	0/0	0%0	0/.07	%0	0%0	0%D			
Natural	% Hydro	۷		47%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	10001	1000/	%_nn1	0/0	00%	%001	00.00			
3-lanes) for	mpervious Area	(ac)		25.04	6.99	5.11	9.97	9.72	6.94	11.95	18.45	17.36	17.36	8.43	8 43	00.8	0.00	0.01 8 7.2		10.00	0.00	11.11			
onditions (8	Drainage	Area (ac)		6.74 C.75	13.2	7.2	14.1	13.4	9.1	16.6	28.3	21.3	21.3	13.1	13.1	8.2	11 8	10.4	17.0	7.14	10 1	tio			
opment Co	Impervious	(II) UIDIAA	010	017	017	216	216	216	216	216	216	216	216	216	216	216	216	216	216	216	216	2			
st-Devel	Length	9	EDED	0000	14-10	1030	2010	1960	1400	2410	3/20	3500	3500	1700	1700	1210	2080	1760	3387	1280	2240	2			
CN - LO	Basin		119-1	140 20	4/6/24	4(1)-20	4(T)-2C	4(1)-38	4(1)-30	4(1)-30	4(1)-30	4(t)-4a(w)	4(t)-4a(e)	4(f)-4b(w)	4(f)-4b(e)	4(f)-5a	4(f)-5b	4(f)-6a	4(f)-6h	4(f)-6c	4(0-7				

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-Development Conditions for Natural Storage Areas	_	100-Yr Runoff (in)	82	54	5.9	5.9	6.0	6.0	5.9	5.7	6.1	6.1	5.9	5.9	6.0	5.9		82	99	19	6.7	
	ts:	25-Yr Runoff (in)	5.5	6	42	4.2	4.3	4.3	4.2	4.1	4.5	4.5	4.2	4.2	4.3	42		63	4 B	4 5	5.0	
	55 Resul	100-Yr Peak Outflow (cfs)	388	68	51	104	70	62	06	159	114	114	22	20	17	75		67	265	12	149	
	TR	25-Yr Peak Outflow (cfs)	291	47	36	74	50	44	63	111	81	81	49	49	56	53		52	193	22	109	
		Tc (hrs)	0.26	0.36	0.21	0.19	0.44	0.25	0.39	0.34	0.45	0.45	0.40	0.40	0.10	0.45		0.55	0.47	0	0.22	
	3	Slope (ft/ft)	0.020	0.003	0.010	0.021	0.004	0.006	0.009	0.006	0.006	0.006	0.011	0.011	0.023	0.016	0.002	0.004	0 008	0.018	0.018	
	ow Concentrated Flo	Difference in Elevation over length (ft)	42	4	12	32	7	7	18	7	11	11	26	26	18	18	2	7	18	14	31	
		Flow Length (ft)	2065	1300	1200	1500	1600	1100	2100	1200	2000	2000	2400	2400	800	1100	1200	2000	2400	800	1700	
	Shall	Surface Description	Unpaved	Unpaved	Unpaved	Unpaved	Unpaved	Unpaved														
		Slope (ft/ft)	0.020	0.020	0.020	0.020	0.020	0.020	0.020	0.020	0.020	0.020	0.020	0.020	0.020	0.020		0.020	0.020	0.020	0.020	
		Difference in Elevation over length (ft)	0.69	0.69	0.69	0.69	0.69	0.69	0.69	0.69	0.69	0.69	0.69	0.69	0.69	0.69		0.69	0.69	0.69	0.69	
	Sheet Flow	2-yr, 24-hr Rainfall: Figure B- 3 (in)	5	2 D	ູ	5	5	S	ŝ	2 2	S S	Q	ŋ	5	5	5		2	5	S	5	
		Length (ft)	34	34	34	34	34	34	34	34	34	34	34	34	34	34		34	34	34	34	
		Surface Description	Smooth Surface		Smooth Surface	Smooth Surface	Smooth Surface	Smooth Surface														
		2099	1334	1234	1534	1634	1134	2134	1634	2034	2034	2434	2434	834	1134	1200	2034	2434	834	1734		
		47.5	13.2	7.23	14.1	13.4	1.7 1.7	10.0	28.3	21.3	21.3	13.1	13.1	8.2	14.8		10.4	47.2	7.7	18.4		
"Q" - Pre-		4(f)-1	4(f)-2a	4(1)-2b	4(1)-20	4(1)-33	4(1)-30	4(1)-30	4(1)-30	4(1)-4a(w)	4(I)-4a(e)	4(r)-4b(w)	4(1)-4b(e)	4(1)-58	4(t)-5D		4(f)-6a	4(f)-6b	4(f)-6c	4(f)-7		
Sheet Flow	Sheet Flow	Sheet Flow	Sheet Flow	heet Flow				Shallov	v Conce	intrated Flow				Door!								
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n Drai	posed inage	Total Length:	Surface	Lenath	2-yr, 24-hr Rainfall	Difference in Flevation	Stone		Flow	Difference	ī		25-Yr	100-Yr	25-							
Are	a (ac)	Path (ft)	Description	Ð	Figure B- 3 (in)	over length (ft)	(ft/ft)	Description	Length (ft)	over length (ft)	Slope (ft/ft)	lc (hrs)	Peak Outflow (cfs)	Dutflow (cfs)	Run (ir							
4	7.5		Smooth Surface	48	5	0.96	0.020	Paved	2065	42	0000	101	307	200	4							
	3.2		Smooth Surface	48	5	0.96	0.020	Paved	1300	4	0.003	0.00	NGC NGC	111								
	.23		Smooth Surface	48	5	0.96	0.020	Paved	1200	12	0.010	0.17	89	- Ha	2							
	4 1		Smooth Surface	48	ъ	0.96	0.020	Paved	1500	32	0.021	0 15	136	174								
- 1º	3.4		Smooth Surface	48	ນ	96.0	0.020	Paved	1600	7	0.004	0.23	114	145	200							
			Smooth Surface	48	S	0.96	0.020	Paved	1100	7	0.006	0 16	6	116	7 4							
=	6.6		Smooth Surface	48	5	0.96	0.020	Paved	2100	18 ·	0.009	0.30	124	158								
	8.3		Smooth Surface	48	ں م	0.96	0.020	Paved	1200	<u> </u>	0.006	0.23	224	288	24							
	2.0		Smooth Surface	48	5	0.96	0.020	Paved	2000	11	0.006	0.29	175	220	17							
	2.0		Smooth Surface	48	ŝ	0.96	0.020	Paved	2000	11	0.006	0.29	175	220	7.7							
			Smooth Surface	48	5	0.96	0.020	Paved	2400	26	0.011	0.34	86	111	99							
			Smooth Surface	48	5	0.96	0.020	Paved	2400	26	0.011	0.34	86	111	6.6							
	2.2		Smooth Surface	48	ۍ م	0.96	0.020	Paved	800	18	0.023	0.10	93	118	7.7							
7	4.œ		Smooth Surface	48	2	0.96	0.020	Paved	1100	18	0.016	0.38	97	124	6.9							
								Paved	1200	2	0.002											
= \$ _	4.0		Smooth Surface	48	S	0.96	0.020	Paved	2000	7	0.004	0.29	94	116	8.4							
311	2		Smooth Surface	48	S	0.96	0.020	Paved	2400	18	0.008	0.34	275	362	5.8							
	-		Smooth Surface	48	5	0.96	0.020	Paved	800	14	0.018	0.10	95	119	7.8							
	4.4		Smooth Surface	48	v	90 0	0000		I COLT				The second se		2							

			Total Required	SMF	Volume (ac-ft)		12.9	4.9	3.4	6.5	6.7	47		0.0	12.8	11.7		5.3		3.8	6.8		4.5	10.5	4.7	6.6	
				25-Year	Volume	(IL-DP)	5.30	1.70	1.10	2.10	2.40	1 60	02.0	2.10	4.40	4.00		1.80		1.20	2.20		1.90	4.80	1 30	2.30	
		lume			Adjusted	(CLO)	N/A	N/A	N/A	N/A	N/A	N/A	VIN	EN.	A/N	N/A	NA	N/A	NIA	N/A	N/A		N/A	N/A	NIA	NA	west.
		uation Vo		Adjusted	Q _o /Q _i	INGIO	N/A	N/A	N/A	N/A	N/A	N/A	NIA		A/N	N/A	NA	N/A	N/A	N/A	N/A		N/A	N/A	N/A	N/A	eas on the
		ear Atten		Actual	Q°/Q Batio	11410	0.73	0.56	0.53	0.54	0.44	0.48	0.51		00.0	0.46	0.46	0.57	0.57	0.60	0.55		0.55	0.70	0.55	0.68	orage are
		25-Ye	Post 25- Year	Peak	Inflow, Q, (CES)	12 221	397	84	68	136	114	92	124	100	472	9/1	175	86	86	93	67		94	275	95	160	e natural st
reas			Pre 25- Year	Peak	Outflow,	10 0 00	291	47	36	74	50	44	63	111	-	81	81	49	49	56	23		52	193	52	109	ished in the
	1000		Required	Ireatment	(ac-ft)		2.1	9.0	0.4	0.8	0.8	0.6	1.0	- -		+	1.4	0.7	0.7	0.5	0.9	1	0.7	1.4	0.5	0.9	be accompl
Storado	oror and a		Impervious	Drainage	Area (ac)		70.0	7.0	5.1	10.0	9.7	6.9	12.0	18.4	17.4	+''-	11.4	8.4	8.4 0	6.0	10.3	t	8.1	16.8	6.3	11.1	assumed to
or Natura			%	Drainage	Area	010	0.03	50.0	17.0	00	0./3	0.77	0.72	0.65	0.81	10:0		0.64	0.64	0.74	0.70	100	0.84	0.30	0.83	0.60	uation was
(8-lanes)	Tractor	Ireaun	Required	Treatment	(in)	•	-		-,	-	_	-	-	t						-	_	-	-,	_	-	-	ent and atten
ements			SMF	Type	(wet/dry)	INICH	INITI	ING									Contraction of the local division of the loc	Concentration of the local division of the l									treatme
e Requir			Bacin	Tvne	2	Posel		Dasono	nasnio	Cidsed	Closed	Closed	Closed	Closed	Closed	Clored	Clocod	Closed	Closed	Closed	כוספנת	Cheed			Closed	Closed	ed since th
Management Facility (SMF) Volume		and the second se	100-Yr Post-Pre	Attenuation	Volume (ac-ft)	22	9.0		- C	0.0	0.0	0.2	4.3	6.8	6.2	63	400	2.2	2.4		1.0	0		2.4	z.4	3.4	e)] are shade
	Ы		Post 100-Yr	Runoff	Volume (ac-ft)	34.4			407	10.01	2.4		12.4	20.3	17.1	17.1	93	200	6.0	110	2	00	20.2	2.00	0.0	13.0	4(1)-4b(e
	ion Volur		Pre 100-Yr	Runoff	(ac-ft)	28.0	202	2.0	20.2	2 4			8.1	13.4	10.9	10.9	64	P 9	41	73	2	71	76.0	200	2.0	10.3	<u>4a(e) and</u>
	Attenuat		Proposed Drainage	Area	(ac)	47.5	13.7	7.0	141	13.4	10		10.0	28.3	21.3	21.3	13.1	13.1	82	14.8	2	10.4	47.2	77		18.4	/e rows (4(1)-
mwater		4	100-	Y.	(t)	0.73	0.65	0.75	0.74	0.76	0.78	240	00	0.72	0.80	0.80	0.71	0.71	0.76	0.74		0.87	0.64	0.87	70.0	1.14	I Ne apor
ed Stor				7		0.61	0.45	0.49	0.49	0.50	0.50	0 40	0.43	0.48	0.51	0.51	0.49	0.49	0.50	0.49		0.68	0.55	0.51	0.50	Notes I	INOIE.
Estimate			Basin			4(f)-1	4(f)-2a	4(f)-2b	4(f)-2c	4(f)-3a	4(f)-3h	A(f) 3c	10.01	4(t)-3d	4(f)-4a(w)	4(f)-4a(e)	4(f)-4b(w)	4(f)-4b(e)	4(f)-5a	4(f)-5b		4(f)-6a	4(f)-6b	4(f)-6c	10.7	1.7.1	

											_	
	Total Req'c SMF Area (ac)	6 20	616	10 51	2.4	8.37		0.99	00.4	5.33	11.20	4.24
	SMF Length (w/ Side Slopes & Berms) (ft)	107	707	1018	210	828		100	034	657	964	584
	SMF Width (w/ Side Slopes & Berms) (ft)	386	380	535	356	440	007	403	040	353	506	316
	Berm Width (ft)	20.00	20.00	20.00	20.00	20.00	00.00	00.02	00.02	20.00	20.00	20.00
	Water Surface Area @ Peak Design Stage	235380	226591	484550	196538	315378	010010	010007	001001	193191	430647	150228
Areas	SMF Length w/ Side Slopes (ft)	681	667	978	621	788	745	201	100	/10	924	544
Storage	SMF Width w/ Side Slopes (ft)	346	340	495	316	400	363	202	000	010	400	276
r Natural	Assumed Side Slope	4.00	4.00	4.00	4.00	4.00	4 00	001	001	100	4.00	4.00
nes) fo	SMF Length (ft)	671	655	996	609	776	703	582	807	100	0 0	536
s (8-la	SMF Width (ft)	336	328	483	304	388	351	201	303	AFD		268
uirement:	Initial Required SMF Area (sq ft)	225213	214653	467010	185432	301261	247023	169482	183992	410500	100001	143/31
rea Regi	Initial Required SMF Area (ac)	5.17	4.93	10.72	4.26	6.92	5.67	3.89	4 22	0.63	00.0	0.00
(SMF) A	Required SMF Volume (ac-ft)	12.9	14.8	32.2	12.8	20.7	17.0	11.7	10.6	19.3		0.0
Facility	Chosen SMF Depth (ft)	2.5	3	e	ო	ę	e	e	2.5	~		7
ted Stormwater Management	Approximate Roadway Elevation (ft, NGVD)	117	73	78	78	78	73	76	65	61	63	20
	SCS Depth to SHWT (ft)	1.5-3.5	3.5-6	9<	9<	9^	9<	9^	1.5-3.5	5	1 5,3	2
	Est. Existing Ground Elevation of SMF Site (ft, NGVD)	06	65	0	02	70	68	20	58	55	55	22
Estime	Basin	4(f)-1	4(f)-2	4(1)-3	4(t)-3d	4(f)- 3c/ 4(f)-3d	4(f)-4	4(f)-4a	4(f)-5	4(f)-6	4(f)-7	

100-year Floodplain Impact Calculations

I-75 Project Development and Environment Study Estimated Alternative Stormwater Management Facility. Floodplains Impact. WPI Seg. No.: 4110141 FAP No.: 0751-120I

Basin	100-YR Floodplain Impacts within the R/W	Alternative Stormwater Management Facility (SMF) I.D.#	Approximate Station Location and Side	Area (ft²) Right	Area (acres)
3		3B	1287+00, Right	95064	2.18
3 & 4	1		1288+00, Left	14734	0.34
3 & 4	2		1286+50, Right	13877	0.32
4		4B	1295+50, Right	85700	1.97
4		4C	1298+50, Left	148936	3.42
5	3		1331+00, Left	4634	0.11
5	4		1332+00, Right	2985	0.07
5		5C	1333+00, Right	8586	0.20
5		5B	1334+00, Right	6315	0.14
8&9	5		1425+00, Left	5762	0.13
8&9	6		1440+00, Right	38123	0.88
8		8C	1422+00, Right	87394	2.01
9		9A	1425+00, Leftt	11272	0.26
9		9B	1427+00, Right	105219	2.42
9	7		1433+00, Left	9349	0.21
10	8		1446+00, Left	12559	0.29
		10A	1446+00, Right	178152	4.09
		10C	1447+00, Right	178152	4 09
15		15B	1597+00+00. Right	178633	4.10
15		15C	1422+00, Right	13474	0.31
17		178	1688±00 Log	5604	0.40
10	l		1000+00, Len	0094	0.13
18		18A	1700+00, Right	219093	5.03
18	12- 1203-	18B	1707+00, Right	327288	7.51
19		19A	1765+00, Left	169465	3.89

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